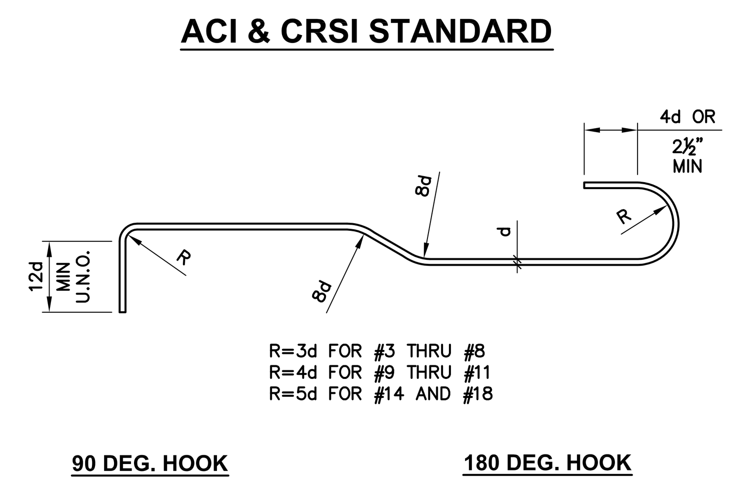
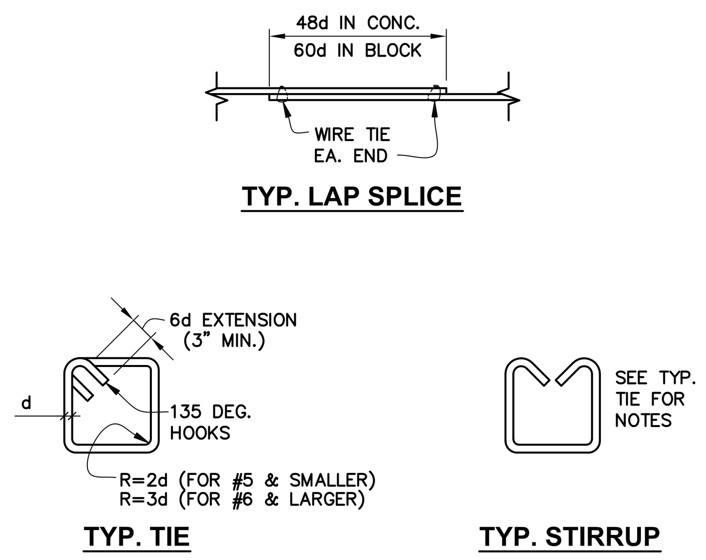


TYP. REBAR HOOKS AND BENDS — 1 / S1.0

SCALE: NO SCALE 1309TYP03200



TYP. REINF. DETAILS — 2 / S1.0

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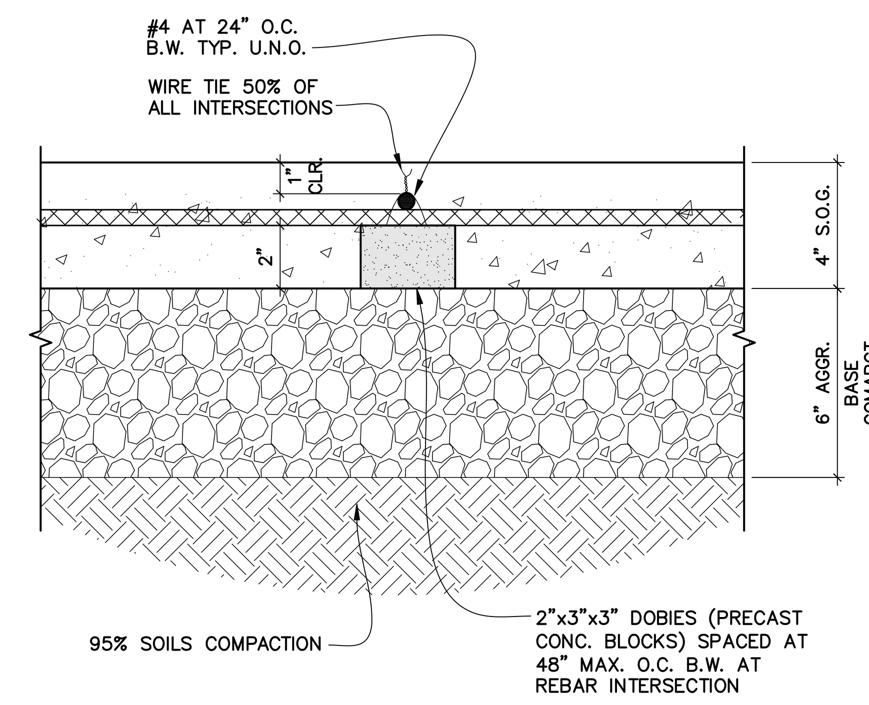
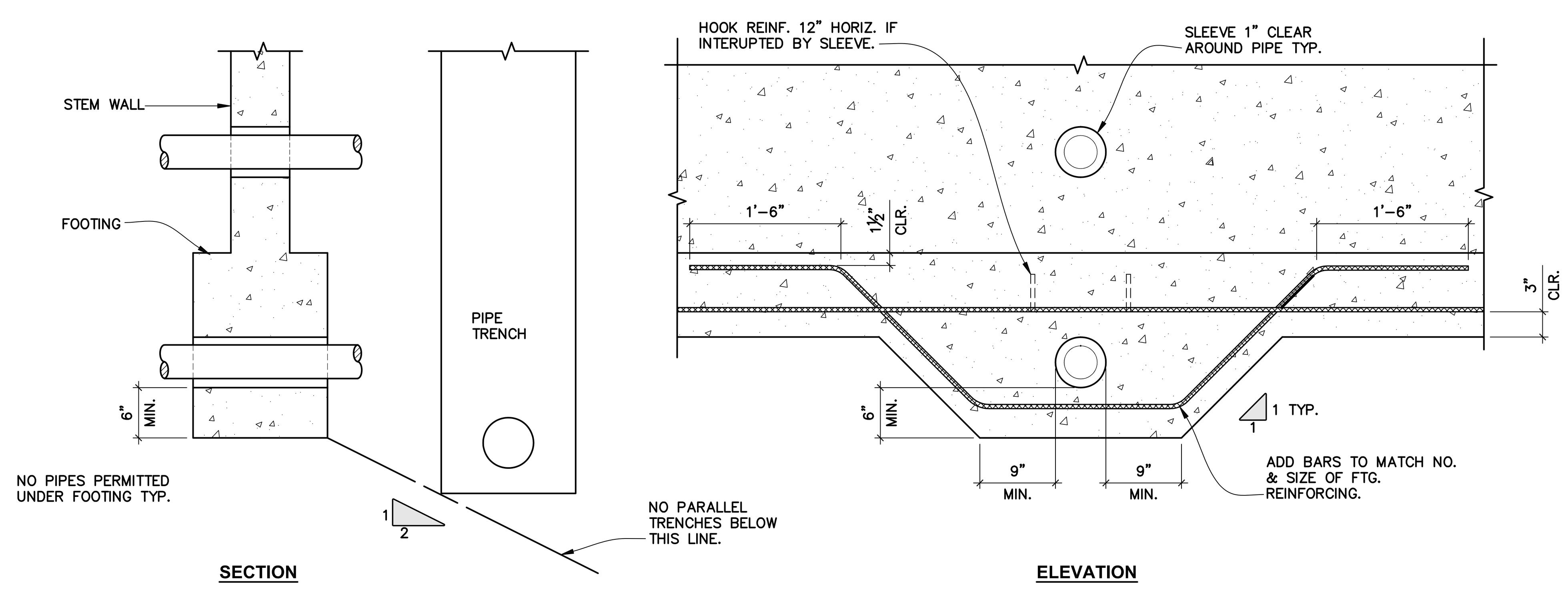


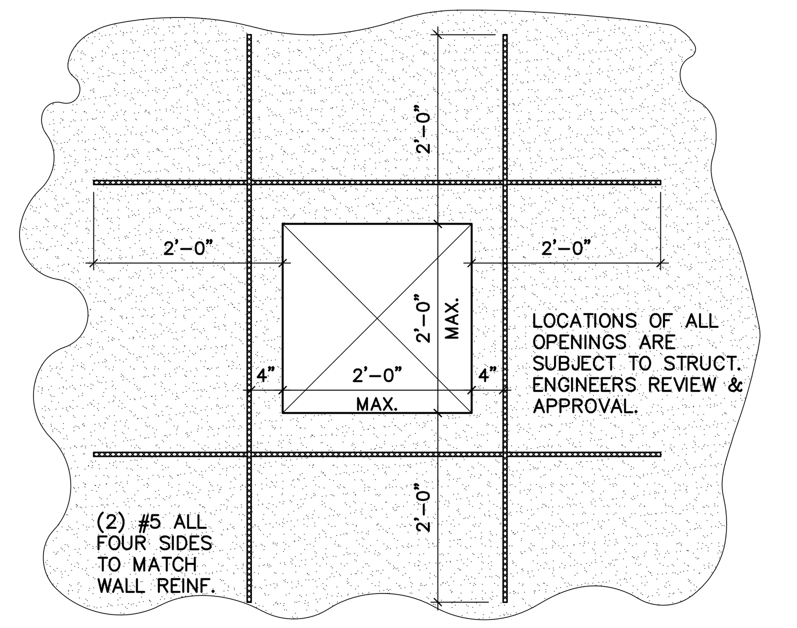
TYP 4" S.O.G. REBAR DOBIE BLOCK SUPPORT DETAIL — 3 / S1.0

SCALE: 3" = 1'–0" 1309TYP03325



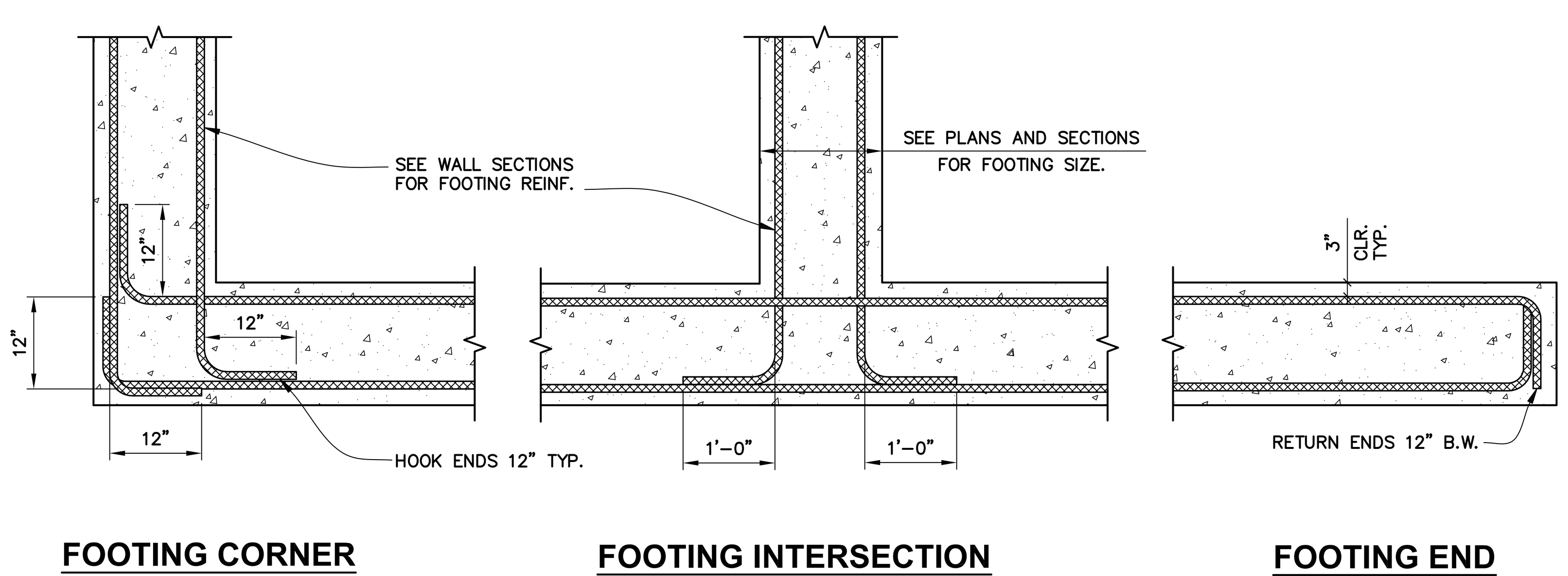
TYP. PIPE PASSAGES AT FOOTINGS — 4 / S1.0

SCALE: NO SCALE 1309TYP03300



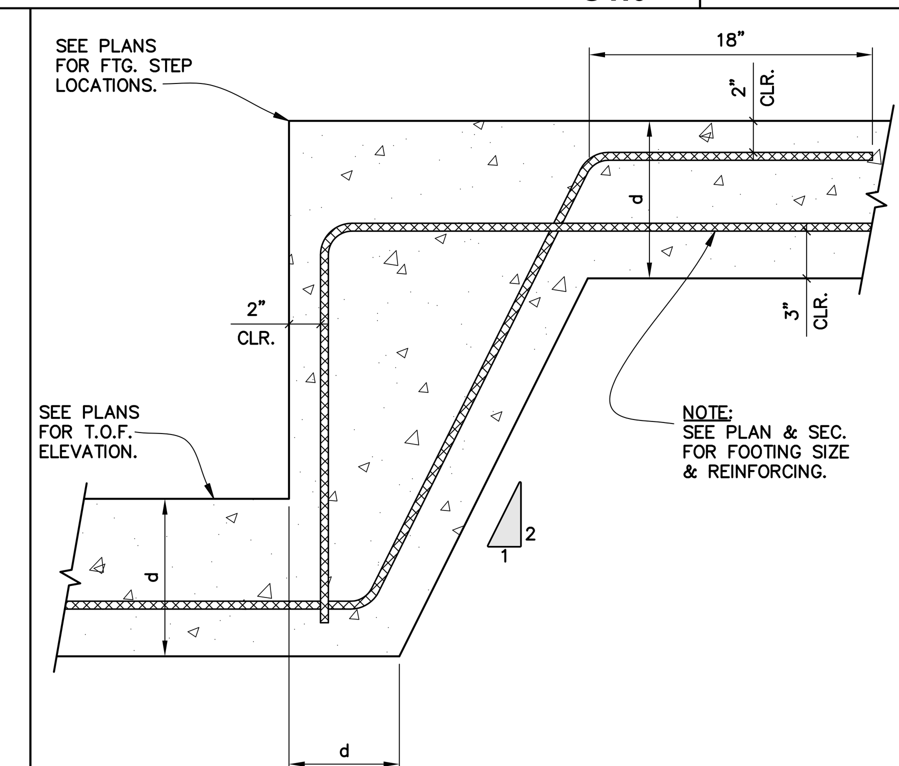
TYP. OPENING IN CONC. WALL — 5 / S1.0

SCALE: 3/4" = 1'–0" 1309TYP04213



TYP. REINF LAPS IN FTG — 6 / S1.0

SCALE: NO SCALE 1309TYP03203



TYP. FOOTING STEP — 7 / S1.0

SCALE: 1 1/2" = 1'–0" 1309TYP03308

KNE Structural Index

Sheet No.	Sheet Title
S1.0	GENERAL NOTES AND TYPICAL DETAILS
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S1.2	TYPICAL DETAILS
S2.0	STRUCTURAL BASEMENT FOUNDATION PLAN
S2.1	STRUCTURAL FIRST FLOOR FRAMING DETAIL
S2.2	STRUCTURAL FIRST FLOOR WALL FRAMING PLAN
S2.3	STRUCTURAL SECOND FLOOR FRAMING PLAN
S2.4	STRUCTURAL SECOND FLOOR WALL FRAMING PLAN
S2.5	STRUCTURAL ROOF FRAMING PLAN
S3.0	PARTIAL STRUCTURAL ELEVATION AND STEEL CONNECTION DETAILS
S4.0	STRUCTURAL BASEMENT DETAILS
S4.1	STRUCTURAL FLOOR FRAMING DETAILS
S4.2	STRUCTURAL FLOOR TO FLOOR DETAILS
S4.3	STRUCTURAL ROOF FRAMING DETAILS
S4.4	STRUCTURAL SITE RETAINING WALL SECTIONS AND DETAILS

TYPICAL NAILING SCHEDULE

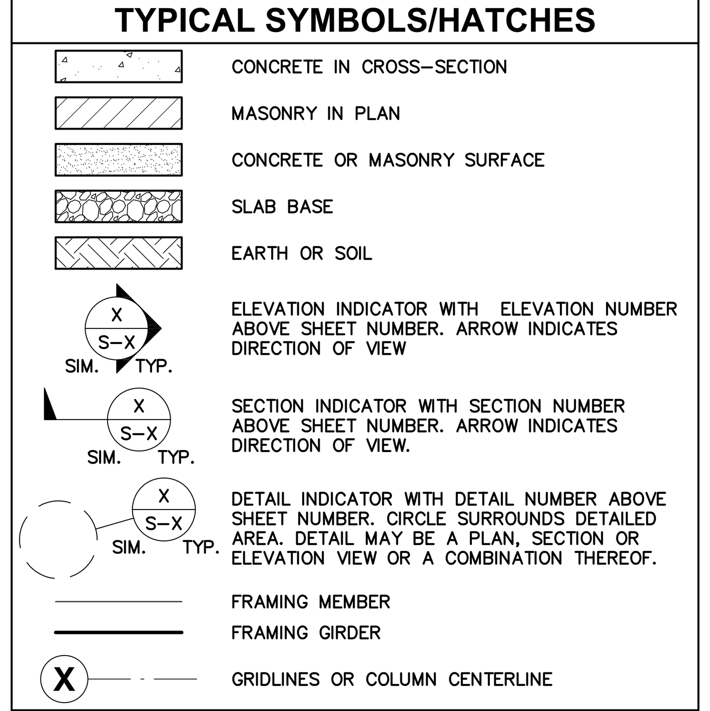
COMMON NAILS ONLY (IBC TABLE 2304.9.1)

	CONNECTION	LOCATION	FASTENING
1	JOIST TO SILL OR GIRDER	TOENAIL	3–8d
2	BRIDGING TO JOIST	TOENAIL EACH END	2–8d
3	1"x6" SUBFLOOR OR LESS TO EACH JOIST	FACE NAIL	2–8d
4	WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST	FACE NAIL	3–8d
5	2" SUBFLOOR TO JOIST OR GIRDER	BLIND AND FACE NAIL	2–16d
6	SOLE PLATE TO JOIST BLOCKING SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	TYPICAL FACE NAIL BRACED WALL PANELS	16d AT 12" O.C. 16d AT 12" O.C.
7	TOP PLATE TO STUD	END NAIL	3–16d
8	STUD TO SOLE PLATE	TOENAIL END NAIL	6–8d 3–16d
9	DOUBLE STUDS	FACE NAIL	16d AT 8" O.C.
10	DOUBLE TOP PLATES	TYPICAL FACE NAIL LAP SPLICE	16d AT 12" O.C. 8–16d
11	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	TOENAIL	3–8d
12	RIM JOIST TO TOP PLATE	TOENAIL	8d AT 6" O.C.
13	TOP PLATES, LAPS AND INTERSECTIONS	FACE NAIL	3–16d
14	CONTINUOUS HEADER, TWO PIECES	12" O.C. ALONG EDGE	16d
15	CEILING JOIST TO PLATE	TOENAIL	3–8d
16	CONTINUOUS HEADER TO STUD	TOENAIL	6–8d
17	CEILING JOISTS, LAPS OVER PARTIONS	FACE NAIL	3–16d
18	CEILING JOISTS TO PARALLEL RAFTERS	FACE NAIL	3–16d
19	TRUSS OR RAFTER TO PLATE	TOENAIL	4–8d
20	1" DIAGONAL BRACE TO EACH STUD AND PLATE	FACE NAIL	2–8d
21	1"x8" SHEATHING TO EACH BEARING	FACE NAIL	3–8d
22	WIDER THAN 1"x8" SHEATHING TO EACH BEARING	FACE NAIL	3–8d
23	BUILT–UP CORNER STUDS	12" O.C.	16d
24	BUILD–UP GIRDER AND BEAMS	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES FACE NAIL AT ENDS AND AT EACH SPLICE	20d 2–20d
25	2" PLANKS	AT EACH BEARING	16d
26	COLLAR TIE TO RAFTER	FACE NAIL	3–10d
27	JACK RAFTER TO HIP	TOENAIL FACE NAIL	3–10d 2–16d
28	ROOF RAFTER TO 2x RIDGE BEAM	TOENAIL FACE NAIL	2–16d 2–16d
29	JOIST TO BAND JOIST	FACE NAIL	3–16d
30	LEDGER STRIP	FACE NAIL	3–16d

TYPICAL ABBREVIATIONS

ALT.	ALTERNATE	K.S.	KING STUD
A.B.	ANCHOR BOLT	LONG.	LONGITUDINAL
ARCH.	ARCHITECT	MAT'L	MATERIAL
BLK.	BLOCK	MAS.	MASONRY
BLK'G	BLOCKING	MAX.	MAXIMUM
BM.	BEAM	M.B.	MACHINE BOLT
B.S.	BOTH SIDES	MECH.	MECHANICAL
BOT.	BOTTOM	MFGR.	MANUFACTURER
B.O.	BOTTOM OF	MIN.	MINIMUM
BTWN.	BETWEEN	(N)	NEW
B.W.	BOTH WAYS	N.S.	NEAR SIDE
C.J.	CONCRETE (OR COLD) JOINT	N.S.A.	NELSON STUD ANCHOR
CL/℄	CENTER LINE	NO.	NUMBER
CLR.	CLEAR	NOM.	NOMINAL
C.M.U.	CONCRETE MASONRY UNIT	O.C.	ON CENTER
COL.	COLUMN	OPN'G	OPENING
CONC.	CONCRETE	PL/℘	PLATE
CONN.	CONNECTION	PROJ.	PROJECTION
CONT.	CONTINUOUS	R=	RADIUS EQUALS
CONTR.	CONTRACTOR	REINF.	REINFORCEMENT
C.S.	COUNTER SINK/CRIPPLE STUD	REQ'D	REQUIRED
DBL.	DOUBLE	SEC.	SECTION
DEG.	DEGREES	SIM.	SIMILAR
DET.	DETAIL	S.O.G.	SLAB ON GRADE
DIAG.	DIAGONAL	STAGG'D	STAGGERED
DO.	DITTO	STD.	STANDARD
EA.	EACH	STIFF.	STIFFENER
ELEV.	ELEVATION	STRUCT.	STRUCTURAL
EQ.	EQUAL	SQ.	SQUARE
(E)	EXISTING	T&B	TOP AND BOTTOM
FIN.	FINISH	T.O.C.	TOP OF CONCRETE
FLR.	FLOOR	T.O. C.M.U.	TOP OF CONCRETE MASONRY UNIT
FND.	FOUNDATION	T.O.F.	TOP OF FOOTING
F.S.	FAR SIDE	T.O.S.	TOP OF STEEL (OR SLAB)
FTG.	FOOTING	T.O.W	TOP OF WALL
GA.	GAUGE	T.J	TOOLED JOINT
GR.	GRADE	TRAN.	TRANSVERSE
HGT.	HEIGHT	TYP.	TYPICAL
HORIZ.	HORIZONTAL	U.N.O.	UNLESS NOTED OTHERWISE
H.S.B	HIGH STRENGTH BOLT	VERT.	VERTICAL
HVY.	HEAVY	w/	WITH
JT.	JOINT	ø	DIAMETER

TYPICAL SYMBOLS/HATCHES



I.B.C. SECTION 1704 SPECIAL INSPECTIONS	REQUIRED YES	NO	N/A
1. CONCRETE:	✓		
2. BOLTS INSTALLED IN CONCRETE:	✓		
3. DUCTILE MOMENT–RESISTING CONCRETE FRAME:			✓
4. REINFORCING STEEL AND PRE–STRESSING STEEL:	✓		
5. WELDING:	✓		
6. HIGH STRENGTH BOLTING:	✓		
7. STRUCTURAL MASONRY:			✓
8. REINFORCING GYPSUM CONCRETE:			✓
9. INSULATING CONCRETE FILL:			✓
10. SPRAY APPLIED FIREPROOFING:			✓
11. PILING, DRILLED PIERS AND CAISSONS:			✓
12. SHOT CRETE:			✓
13. SPECIAL GRADING, EXCAVATION AND FILLING:	✓		
14. SPECIAL CASES (IF YES, CONTACT STRUCTURAL ENGINEER)		✓	
SPECIAL INSPECTION REQUIRED AT ALL DRILLED AND EPOXIED HOLDOWN LOCATIONS.			

GENERAL NOTES

1. GENERAL:

A. THE FOLLOWING GENERAL NOTES SHALL APPLY TO ALL STRUCTURAL DRAWINGS UNLESS SPECIFICALLY SHOWN OR NOTED.

B. ALL WORKMANSHIP AND MATERIALS SHALL CONFORM TO THE 2006 INTERNATIONAL BUILDING CODE (IBC), WASHOE COUNTY BUILDING ORDINANCES, AND REFERENCED STANDARDS OF THE AMERICAN SOCIETY FOR TESTING MATERIALS (ASTM), THE AMERICAN CONCRETE INSTITUTE (ACI), AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC), THE AMERICAN IRON AND STEEL INSTITUTE (AISI), THE AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE), AND THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC).

C. ALL WORK SHALL CONFORM TO THE PLANS, AND THESE GENERAL NOTES IN ALL RESPECTS, AND SHALL BE SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER. APPROVAL TO DEVIATE FROM THE PLANS MUST BE OBTAINED IN WRITING FROM THE STRUCTURAL ENGINEER.

D. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION, INCLUDING ALL SHORING, ERECTION, BRACING, ETC.; AND ALL JOB SITE SAFETY.

2. DESIGN LOADS:

THIS STRUCTURE HAS BEEN DESIGNED FOR THE FOLLOWING LOADS IN ACCORDANCE WITH THE 2006 IBC AND WASHOE COUNTY BUILDING ORDINANCES.

A. VERTICAL LOADS: ROOF – 20psf SNOW LOAD (NO REDUCTIONS), 40psf FLOOR LIVE LOAD, DEAD LOAD AS REQUIRED.

B. LATERAL LOADS: WIND – 100mph (3–SECOND GUST) BASIC WIND SPEED PER WASHOE COUNTY AND THE 2006 IBC, EXPOSURE C, WITH AN IMPORTANCE FACTOR OF 1.0; SEISMIC – 2006 IBC SEISMIC DESIGN CATEGORY D IN ACCORDANCE WITH THE FORMULA $V=C_SW$; WHERE $C_S=0.166$, AND W= EFFECTIVE WEIGHT OF STRUCTURE.

C. FOUNDATIONS: FOUNDATIONS ARE DESIGNED FOR A SAFE ALLOWABLE BEARING PRESSURE OF 2,500 PSF FOR TYPICAL SPREAD FOOTINGS PER SOILS REPORT BY NORTECH GEOTECHINICAL/CIVIL CONSULTANTS, LTD. DATED NOVEMBER 16, 2007.

3. EARTHWORK:

A. BUILDING IS TO BE SUPPORTED DIRECTLY ON PROPERLY ENGINEERED FILL MATERIAL OR FIRM NATIVE SOILS.

B. ALL REQUIRED ADDITIONAL IMPORTED STRUCTURAL FILL MATERIAL SHALL BE "WELL–GRADED" AND SHALL MEET THE FOLLOWING SPECIFICATIONS:

SIEVE SIZE	PERCENT PASSING BY WEIGHT
2–INCH	100%
1½–INCH	90% – 100%
¾–INCH	50% – 80%
No. 4	24% – 40%
No. 16	20% – 24%
No. 100	0% – 4%
No. 200	0% – 2%
LIQUID LIMIT	35 MAXIMUM
PLASTIC INDEX	15 MAXIMUM

C. PLACE ALL FILL AND BACKFILL IN 6 INCH COMPACTED LIFTS, MOISTURE CONDITION AND COMPACT TO 95% OF MAXIMUM DRY DENSITY.

D. ALL EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 24" BELOW FINISH GRADE. AFTER FOOTING EXCAVATION, AND PRIOR TO PLACING FOOTING REINFORCING; SCARIFY, MOISTURE CONDITION, AND RE–COMPACT TO 95% OF MAXIMUM DRY DENSITY. FOOTINGS SHALL BE HAND CLEANED JUST PRIOR TO PLACING OF FOOTING CONCRETE. ALL FOOTINGS SHALL BE FORMED AS SHOWN.

E. BACKFILL AROUND FOOTINGS AND STEM WALLS SHALL BE PLACED IN 6 INCH LIFTS AND COMPACTED TO 95% MAXIMUM DRY DENSITY.

F. ALL SLABS SHALL BE UNDERLAIN WITH NEVADA DEPARTMENT OF TRANSPORTATION (NDOT) TYPE 2 CLASS B AGGREGATE BASE. BASE SHALL BE PLACED IN ONE LIFT AND COMPACTED TO 95% OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

4. CONCRETE:

A. REINFORCED CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF THE 2006 IBC AND ACI STANDARD 318–08.

B. CONCRETE SCHEDULE FOR FOUNDATIONS:

INTERIOR SLABS–ON–GRADE, WALLS AND FOUNDATIONS	CLASS	28–DAY STRENGTH	MAXIMUM SLUMP	AIR CONTENT	MAX. SHRINKAGE	MAX. AGGREGATE
	"A"	3,500 PSI	3 INCHES	–	0.055%	¾ INCH

C. AGGREGATE SHALL CONFORM TO ASTM C33. SEE SPECIFICATIONS.

D. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. ALL REINFORCING STEEL SHALL BE ACCURATELY LOCATED IN THE FORMS AND HELD RIGIDLY IN PLACE BEFORE AND DURING THE PLACEMENT OF CONCRETE BY MEANS OF PROPER WIRE SUPPORTS. THESE SUPPORTS SHALL BE STANDARD CHAIRS AS SHOWN IN THE MANUAL OF STANDARD PRACTICE, PUBLISHED BY CRSI, OR APPROVED EQUAL.

E. USE PRECAST CONCRETE BLOCKS FOR SUPPORTING FOOTING AND SLAB–ON–GRADE REINFORCING.

F. ALL REINFORCING BENDS SHALL BE IN ACCORDANCE WITH ACI STANDARD HOOKS AND BENDS AS SHOWN IN THE ACI MANUAL OF STANDARD PRACTICE.

G. CONCRETE COVERAGE FOR REINFORCING STEEL SHALL BE THE CLEAR DISTANCE TO THE EXTERIOR FACE OF THE BAR AS FOLLOWS:
- FOUNDATIONS POURED AGAINST THE GROUND – 3 INCHES.
- FORMED SURFACES EXPOSED TO THE GROUND OR WEATHER – 1½ INCHES #5 AND SMALLER, 2" INCHES LARGER THAN #5.
- FORMED SURFACES NOT EXPOSED TO THE GROUND OR WEATHER – 1 INCH.
- INTERIOR SLABS–ON–GRADE – 1 INCH AND 1½ INCHES CLEAR OF TOP AS NOTED ON PLANS.

H. REINFORCING SHOWN OR NOTED CONTINUOUS SHALL RUN IN AS LONG A LENGTH AS PRACTICAL. LAP BARS 48 DIAMETERS UNLESS SHOWN OR NOTED OTHERWISE. ALL ADJACENT LAPS SHALL BE STAGGERED 6 FEET. ALL REINFORCING IS INTENDED TO RUN CONTINUOUS TYP. U.N.O.

I. ALL EXPOSED CONCRETE EDGES SHALL BE TOOLED OR CHAMFERED ¾ INCH UNLESS SPECIFICALLY SHOWN OR NOTED.

J. ALL CONCRETE SHALL BE INTERNALLY VIBRATED INTO PLACE.

K. PROVIDE COLD WEATHER AND HOT WEATHER PROTECTION PER ACI 305 & 306.

5. ROUGH CARPENTRY:

A. ALL DIMENSIONED FRAMING LUMBER SHALL BE DOUGLAS FIR LARCH. ALL LUMBER SHALL BE AIR SEASONED WITH A MOISTURE CONTENT FOR ALL LUMBER NOT EXCEEDING 19% AT THE TIME OF INSTALLATION. ALL PLATES, JOISTS, BEAMS, HEADERS, POSTS, AND LEDGERS SHALL BE NO. 1, AND ALL OTHER MEMBERS SHALL BE NO. 2 AND BETTER.

B. NAILING SHALL BE IN ACCORDANCE WITH IBC TABLE 2304.9.1, UNLESS NOTED OTHERWISE ON THE DRAWINGS.

C. PROVIDE SIMPSON CONNECTORS WHERE SHOWN ON THE DRAWINGS. INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS USING MANUFACTURER'S RECOMMENDED NAILS OR BOLTS, IN ORDER TO OBTAIN FULL LISTED LOAD VALUE.

D. BOLTS SHALL BE ASTM A307.

E. CUT FRAMING MEMBERS SQUARE AND TO ACCURATE LENGTH TO OBTAIN FULL BEARING AT JOINTS. ERECT PLUMB AND LEVEL AND TO ACCURATE LINE. PROVIDE NAILING SHOWN IN ACCORDANCE WITH MINIMUM NAILING SCHEDULE REQUIREMENTS. SELECT MEMBERS SO THAT KNOTS OR DEFECTS DO NOT INTERFERE WITH JOINTS OR NAILING. REPLACE ALL NAIL SPLIT MEMBERS.

6. STRUCTURAL STEEL:

A. ALL STRUCTURAL STEEL SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, AND PLATES SHALL CONFORM TO ASTM A36 (U.N.O.); STEEL TUBING – ASTM A500, GRADE B; BOLTS AND ANCHOR BOLTS – A36 OR A307 (U.N.O.).

B. ALL DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF THE AISC SPECIFICATIONS.

C. ALL STRUCTURAL AND MISCELLANEOUS STEEL WORK, EXCEPT STEEL TO BE EMBEDDED IN CONCRETE OR FIELD WELDED, SHALL BE SHOP PRIME PAINTED AND TOUCHED UP IN THE FIELD AFTER ERECTION.

D. ALL WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY'S (AWS) SPECIFICATIONS FOR THE MATERIAL BEING WELDED. WELDING SHALL BE PERFORMED ONLY BY AWS CERTIFIED WELDERS.

E. ALL HIGH STRENGTH BOLTS SHALL BE ASTM A325–SC.

7. GLUED-LAMINATED BEAMS:

A. GLUED–LAMINATED BEAMS SHALL BE 24F–V4 UNLESS NOTED OTHERWISE, SEE DRAWINGS. GLUE WITH WATER RESISTANT GLUE FOR WET CONDITIONS OF USE. CAMBER BEAMS PER DRAWINGS.

B. BEAMS SHALL BE INDUSTRIAL APPEARANCE GRADE.

C. ALL ENDS SHALL BE MILL SEALED, INDIVIDUALLY WRAPPED FOR SHIPMENT, AND PROTECTED DURING CONSTRUCTION.

D. BEAMS SHALL BE ACCURATELY CUT BY THE MANUFACTURER, AND FITTED PER DETAILS. MIS–CUT OR POORLY FITTED JOINTS WILL NOT BE ACCEPTABLE. JOB CUT BEAMS WILL NOT BE ALLOWED.

E. PROVIDE AITC CERTIFICATION INDICATING THAT FABRICATION CONFORMS WITH U.S. PRODUCT STANDARD 56.

8. ENGINEERED LUMBER:

A. ALL ENGINEERED LUMBER SHALL BE BY iLEVEL™ TRUS–JOIST®, A WEYERHAEUSER COMPANY. USE 1.9E LVL FOR BEAMS AND WALL STUDS, 2.0E PSL FOR BEAMS AND COLUMNS, AND 1.3E LSL FOR RIM BOARDS.

B. JOISTS SHALL BE STANDARD MEMBERS AS MANUFACTURED BY iLEVEL™ TRUS–JOIST®.

C. ALL iLEVEL™ TRUS–JOIST® BLOCKING SHALL BE AS INDICATED ON THE DRAWINGS.

D. SEE DETAILS FOR WEB STIFFENERS AND CONNECTIONS.

E. INSTALL MEMBERS PER MANUFACTURER'S INSTRUCTIONS. PROVIDE TEMPORARY BRACING AS NEEDED. CAREFULLY LOCATE AND ALIGN PRIOR TO SECURING IN PLACE. DO NOT OVERSTRESS OR DAMAGE MEMBERS BY IMPROPER LIFTING PROCEDURES.

9. PREFABRICATED WOOD TRUSSES:

A. SEE ROOF FRAMING PLANS FOR LAYOUT AND LOADS.

B. STRUCTURAL DESIGN SHALL BE BY THE MANUFACTURER.

C. ERECTION OF THE ROOF STRUCTURE SHALL BE BY THE CONTRACTOR.

D. SHOP DRAWINGS AND CALCULATIONS SHALL BE SUBMITTED BY THE TRUSS MANUFACTURER FOR REVIEW BY THE STRUCTURAL ENGINEER.

10. SHEATHING AND FASTENERS:

A. SHEATHING SHALL CONFORM STRUCTURALLY TO THE APA STANDARDS, AND TO U.S. PRODUCT STANDARD PS–1 (OR PS–2 FOR O.S.B.). ALL PIECES SHALL BE GRADE STAMPED.

B. ALL ROOF SHEATHING SHALL BE EXPOSURE 1, 5/8", WITH EXTERIOR GLUE, APA GRADE (40/20), AND GRADE STAMPED.

C. ALL FLOOR SHEATHING SHALL BE 1⅛" T&G STURD–I–FLOOR, APA GRADE (48/24) AND GRADE STAMPED.

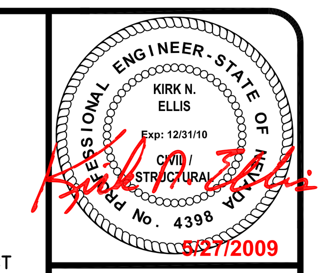
D. ALL EXTERIOR WALL SHEATHING SHALL BE EXPOSURE 1, ½" O.S.B. WITH EXTERIOR GLUE, APA GRADE (32/16), AND GRADE STAMPED.

E. SHEATHING OVER WOOD FRAMING SHALL BE NAILED W/ 10d COMMON x 2⅜" PLYWOOD WIRE NAILS AT THE ROOF AND 10d COMMON x 2¼" PLYWOOD WIRE NAILS AT THE WALLS, AS SHOWN ON THE DRAWINGS, TYP. U.N.O. FLOOR SHEATHING SHALL BE NAILED W/ 10dx3" COMMON SCREW NAILS. ALL NAILS FOR STRUCTURAL CONNECTIONS SHALL BE "COMMON" U.N.O. ALL FLOOR SHEATHING SHALL BE FULLY "GLUED AND SCREWED", TYP. U.N.O.

NOTE:
TYPICAL DETAILS ARE NOT USUALLY CUT OR REFERENCED. THEY ARE INTENDED TO APPLY IN ALL SUCH SIMILAR CONDITIONS, WHETHER REFERENCED OR NOT.

NOTE:
COORDINATE ALL DIMENSIONS, CONDITIONS AND DETAILS WITH THE ARCHITECTURAL DRAWINGS. REPORT ANY DISCREPANCIES TO STRUCTURAL ENGINEER. TYP. U.N.O.

ISSUED FOR BUILDING DEPARTMENT PLAN CHECK
May 27, 2009



KNE
KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST., #425 RENO, NV 89501
PHONE: (775)322-7723 FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

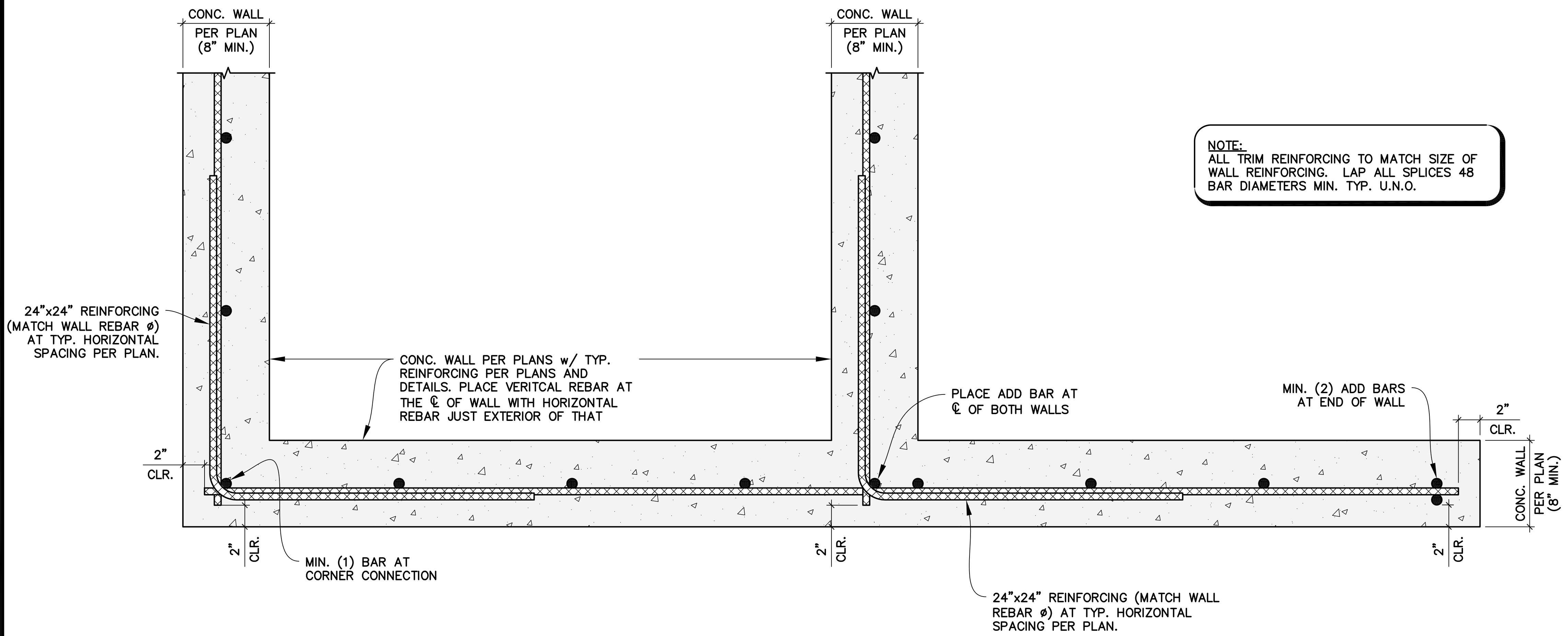
Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:
1. 08/31/2009 ARCH'L COORD.
2. 08/31/2009 BUILDING DEPT.
3. 09/10/2009 BUILDING DEPT.

DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
DESIGNED: TRH/KNE
CHECKED: KNE
KNE JOB NO.: 1309

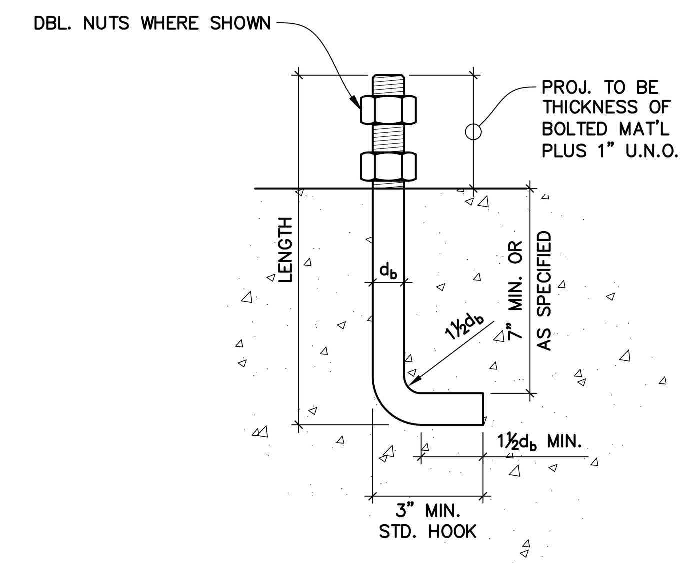
SHEET TITLE: GENERAL NOTES AND TYPICAL DETAILS

SHEET NO. S1.0 OF 15 SHEETS



NOTE:
ALL TRIM REINFORCING TO MATCH SIZE OF WALL REINFORCING. LAP ALL SPLICES 48 BAR DIAMETERS MIN. TYP. U.N.O.

TYP. CONC. WALL REINFORCING — 1/S1.1
SCALE: NO SCALE — 1309TYP04201



TYP. (L) ANCHOR BOLT A.B. — 2/S1.1
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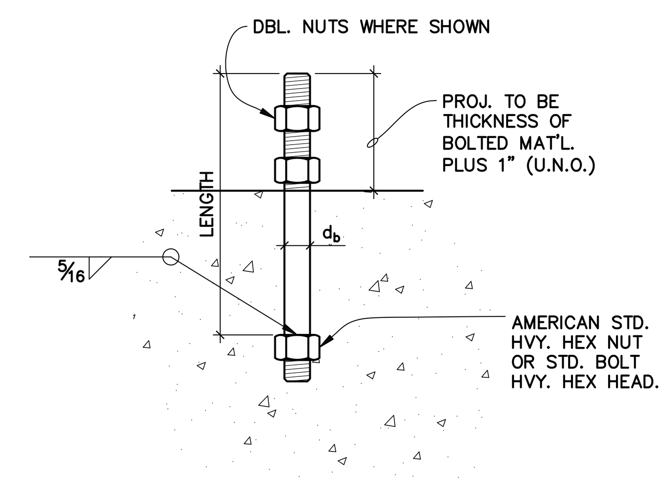
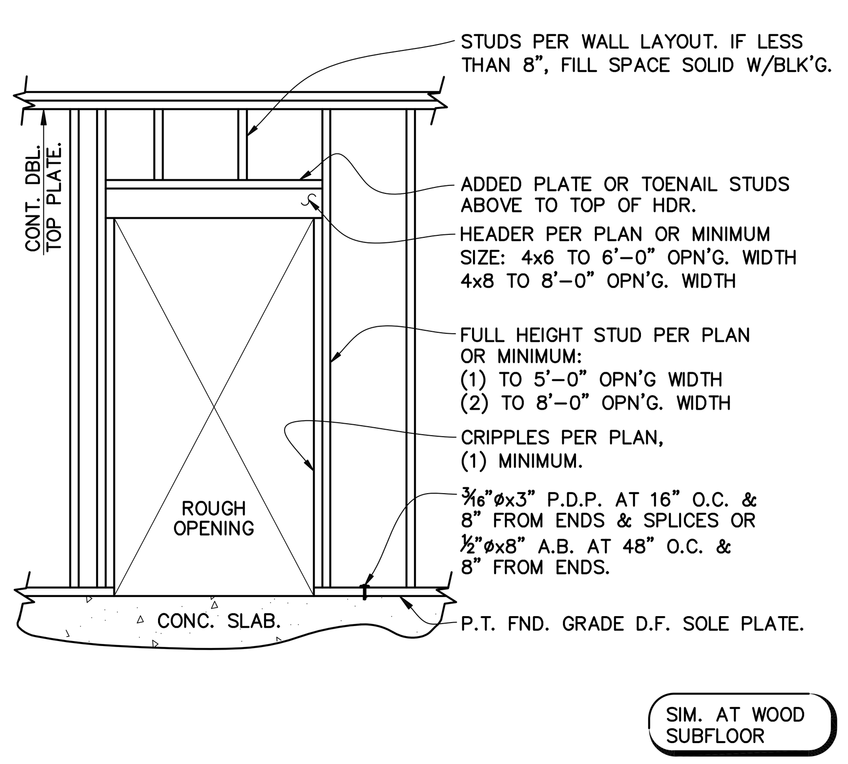
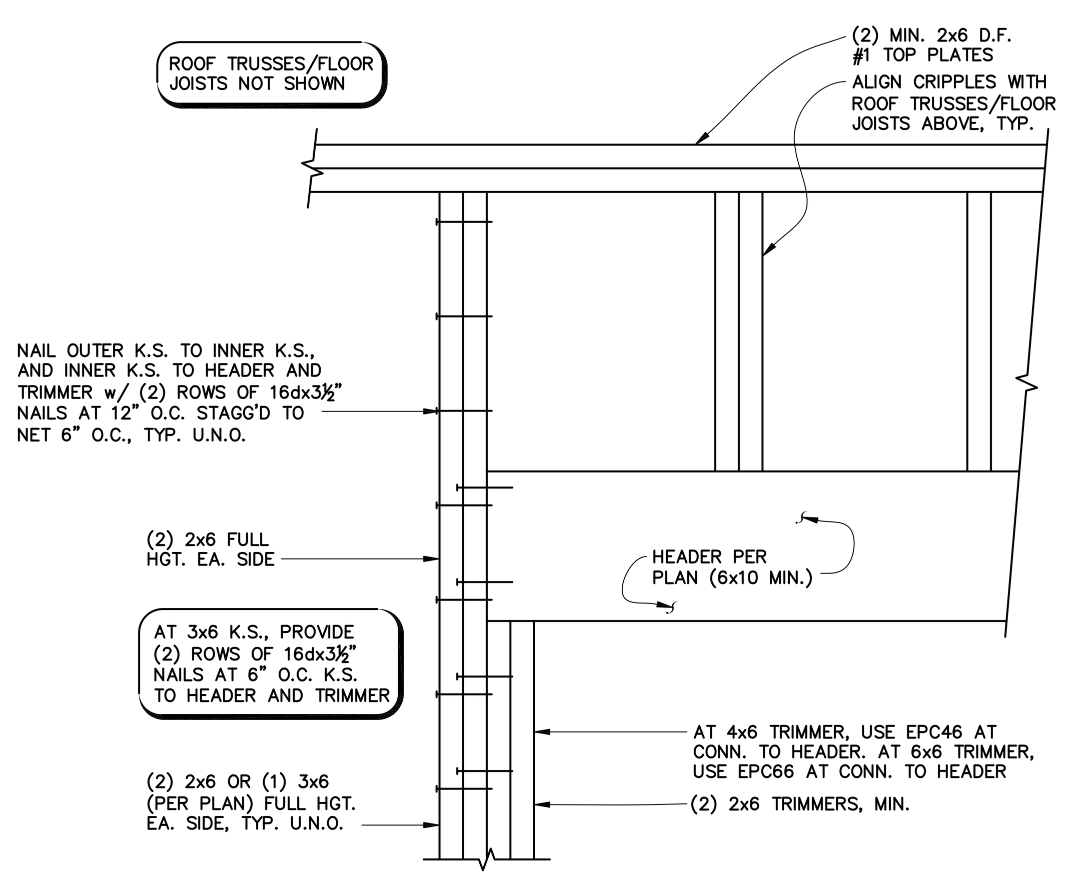


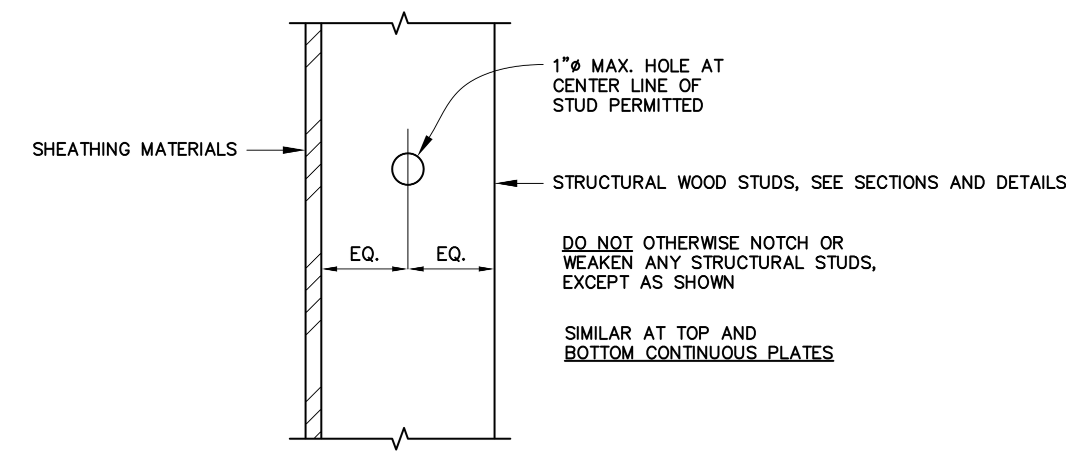
TYP. HEAVY-HEX ANCHOR BOLT (A.B.) — 3/S1.1
SCALE: 3" = 1'-0" — 1309TYP06502



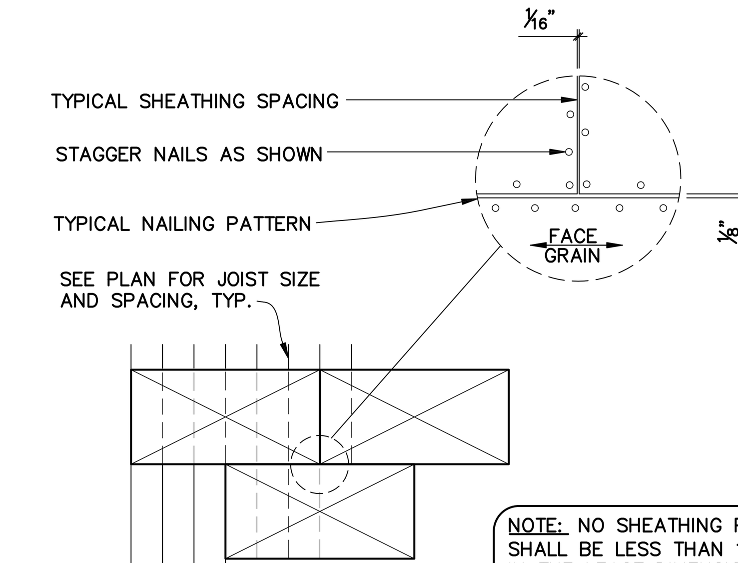
TYPICAL OPENING FRAMING NON-BEARING WOOD WALL — 4/S1.1
SCALE: NO SCALE — 1309TYP06103



TYP. EXTERIOR HEADER — 5/S1.1
SCALE: 1 1/2" = 1'-0" — 1309TYP06102

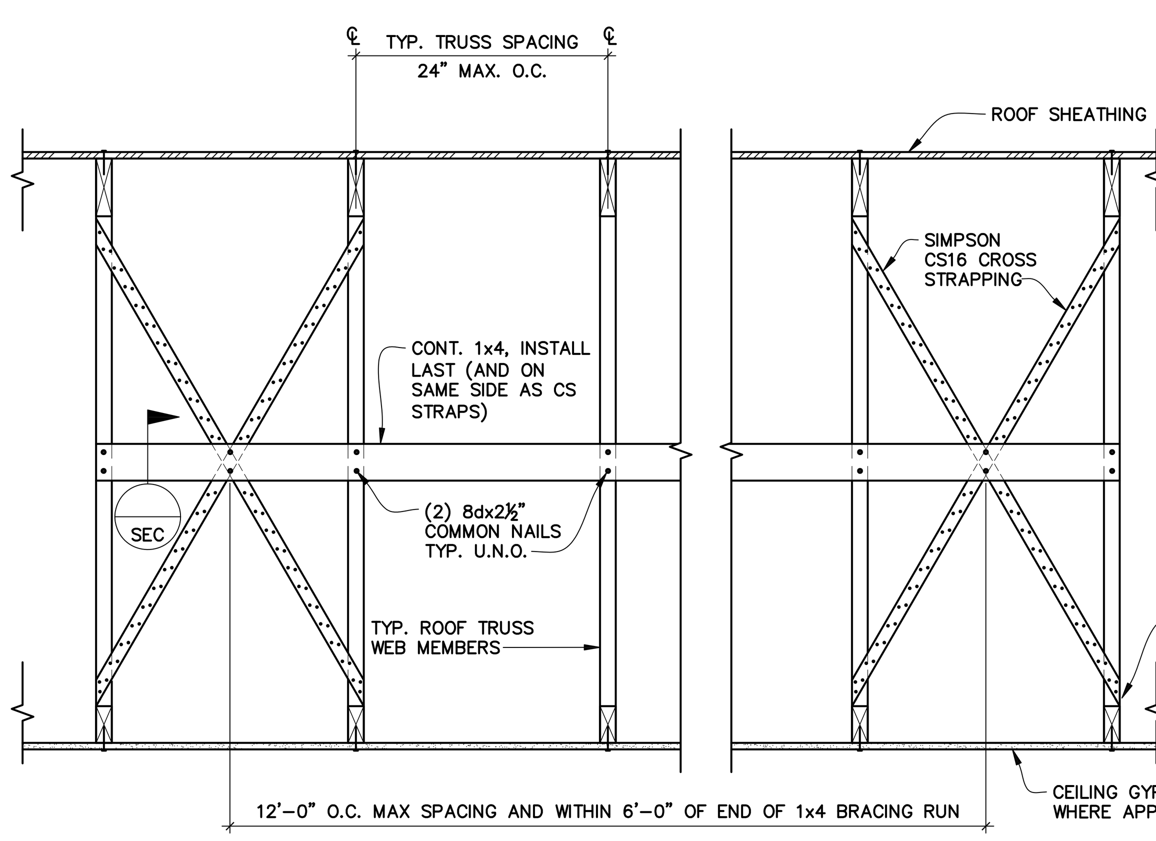


TYP. HOLE IN WOOD STUD — 6/S1.1
SCALE: 3" = 1'-0" — 1309TYP06111

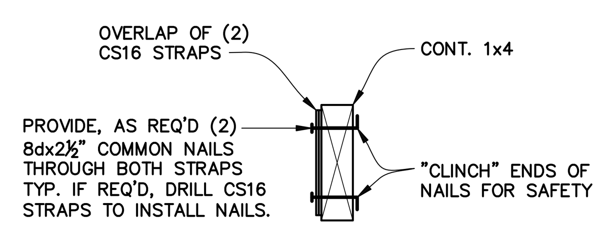


NOTE: NO SHEATHING PANEL SHALL BE LESS THAN 1'-0" IN THE LEAST DIMENSION, OR LESS THAN FOUR SQUARE FEET IN AREA.

TYP. SHEATHING SPACING AND NAILING — 7/S1.1
SCALE: 3" = 1'-0" — 1309TYP06105

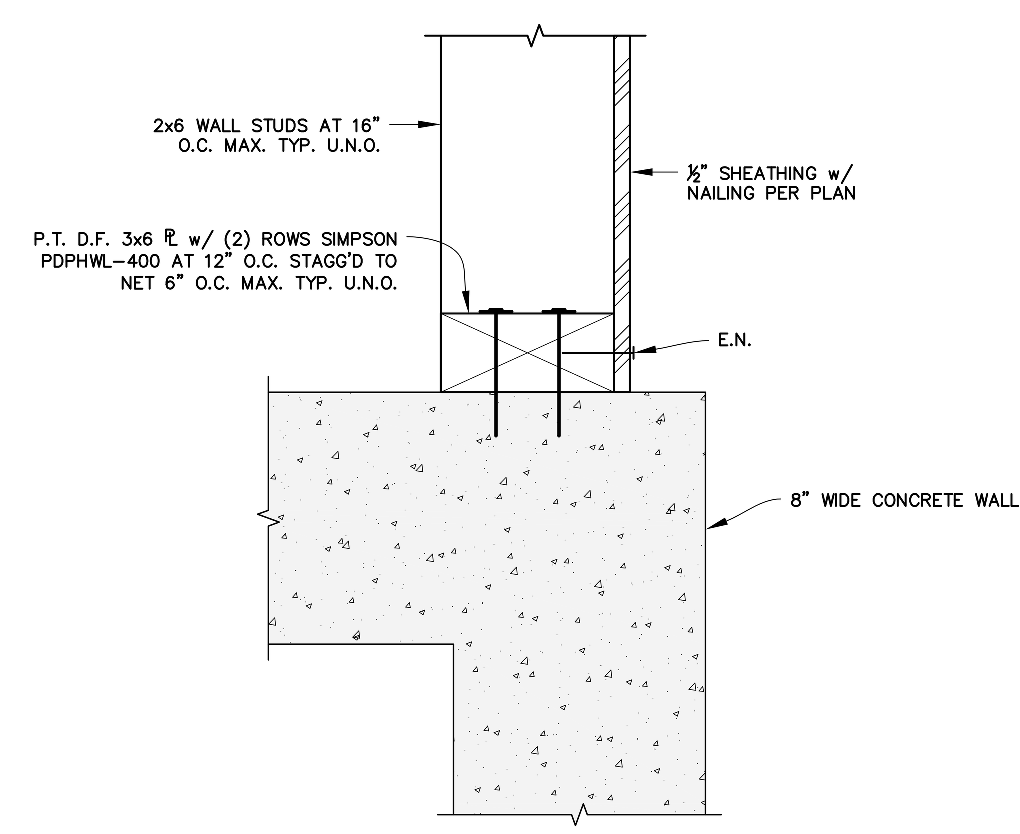


1. THIS DETAIL SHALL BE APPLIED TYPICALLY AT ANY TRUSS WEB MEMBER (OR UN-BRACED BOTTOM CHORD) THAT CLEAR SPANS 6'-0" OR MORE, OR AS NOTED BY TRUSS DESIGNER ON TRUSSES OR BY WHICH REQUIREMENT IS MORE STRINGENT.
2. CONT. 1x4's SHALL BE SPACED AT NOT MORE THAN 6'-0" MAX. O.C. POSITION 1x4's TO CREATE EQUAL UNBRACED LENGTHS OF TRUSS WEBS ON EITHER SIDE OF 1x4'S (e.g.: MID-SPAN OR THIRD POINTS)
3. TRUSS MANUFACTURER SHALL CLEARLY IDENTIFY ON THE ACTUAL TRUSSES WHERE PERMANENT BRACING IS REQUIRED (e.g.: "PURPLE TAG") SO THAT BOTH THE CONTRACTOR AND FIELD INSPECTOR CAN READILY NOTE SAID BRACING LOCATIONS DURING CONSTRUCTION.
4. PRE-DRILL TRUSS WEB'S AS REQ'D TO PREVENT WOOD SPLITTING.

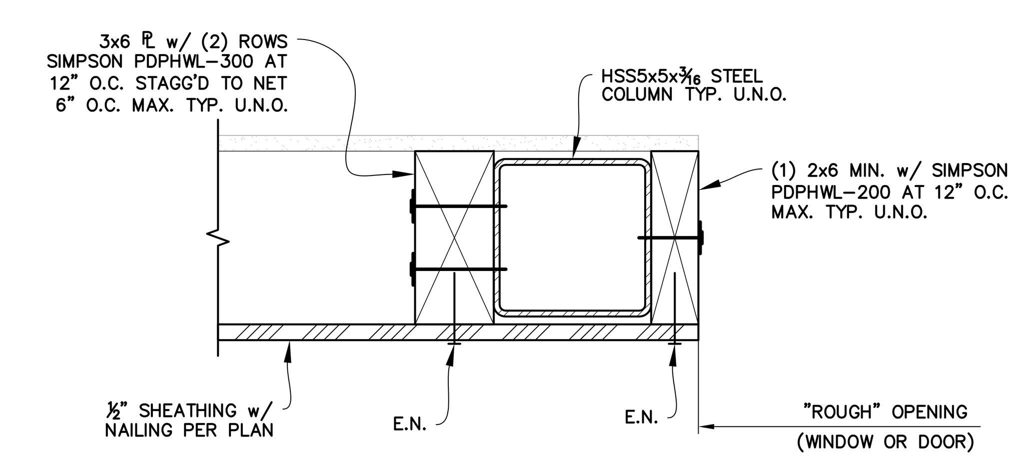


SECTION

TYPICAL TRUSS WEB BRACING REQUIREMENTS — 8/S1.1
SCALE: 1" = 1'-0" — 1309TYP06505



TYP. WOOD WALL TO CONCRETE WALL CONNECTION PLAN VIEW DETAIL — 9/S1.1
SCALE: 3" = 1'-0" — 1309DT52



TYP. STEEL COLUMN AT OPENINGS - PLAN VIEW — 10/S1.1
SCALE: 3" = 1'-0" — 1309DT53

NOTE:
TYPICAL DETAILS ARE NOT USUALLY CUT OR REFERENCED. THEY ARE INTENDED TO APPLY IN ALL SUCH SIMILAR CONDITIONS, WHETHER REFERENCED OR NOT.

NOTE:
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May 27, 2009

KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST., #425 RENO, NV 89501
PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

Professional Engineer State of Nevada — KIRK N. ELLIS — Exp: 12/31/10 — CIVIL / STRUCTURAL — No. 4398 — 5/27/2009

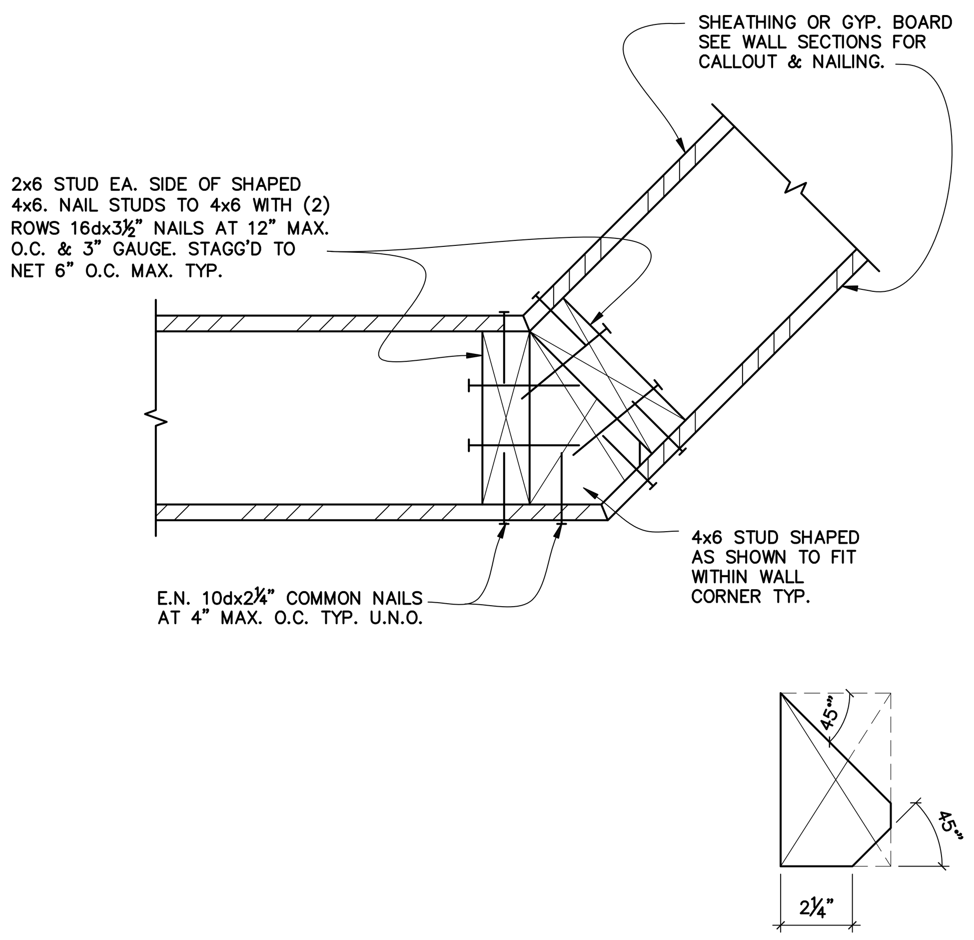
Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:

DATE	05/27/2009
SCALE	AS SHOWN
DRAWN	BLW
DESIGNED	TRH/KNE
CHECKED	KNE
KNE JOB NO.	1309

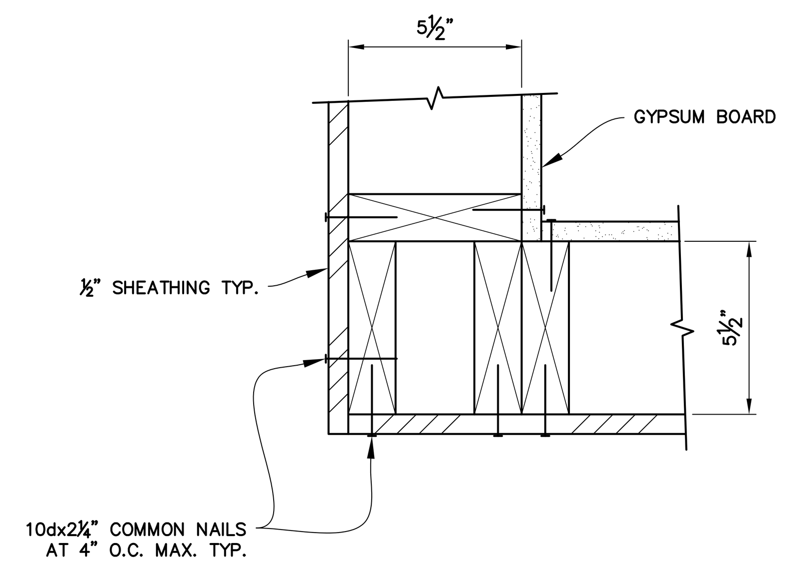
SHEET TITLE: TYPICAL DETAILS

SHEET NO. **S1.1** OF 15 SHEETS



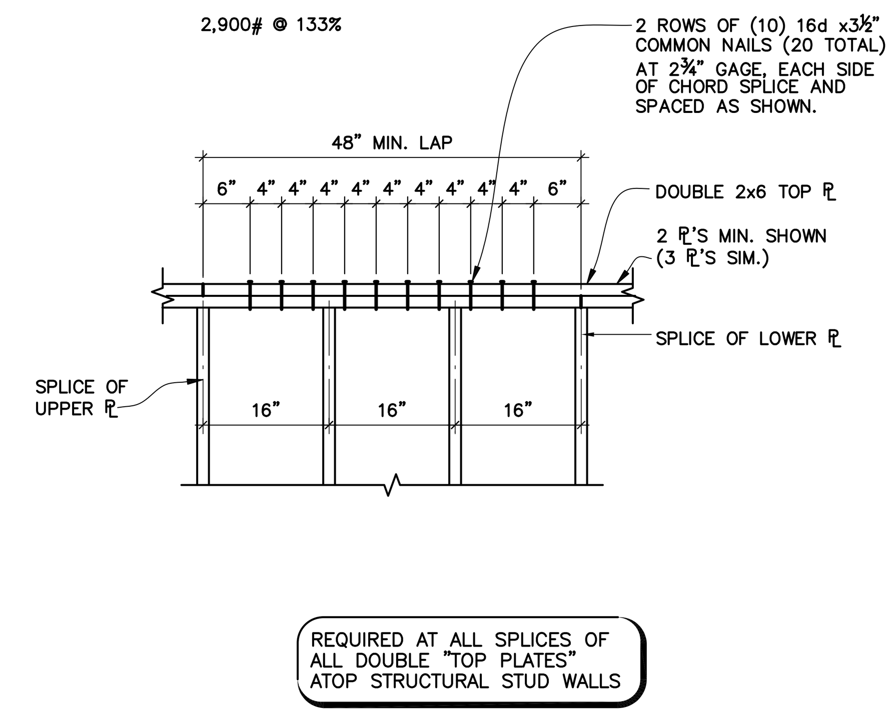
TYPICAL 45° CORNER WALL FRAMING DETAIL — 1 / S1.2

SCALE: 3" = 1'-0" 1309TYP06107



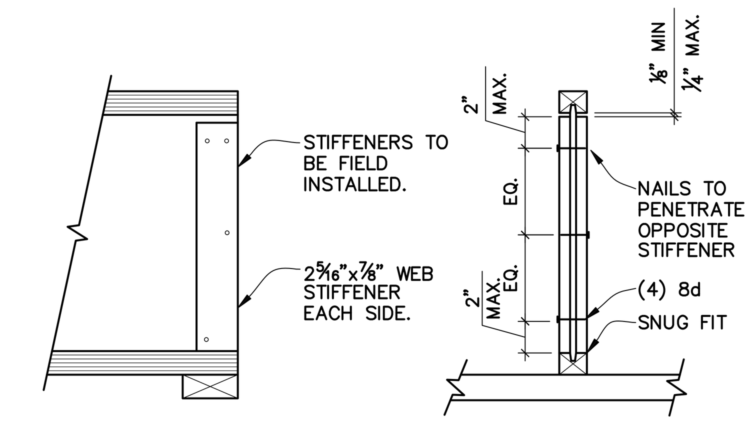
TYPICAL 2x6 STUD WALL FRAMING CORNER DETAIL — 2 / S1.2

SCALE: 3" = 1'-0" 1309TYP06106



TYP. TOP PLATE SPLICE DETAIL — 3 / S1.2

SCALE: 3/4" = 1'-0" 1309TYP06191

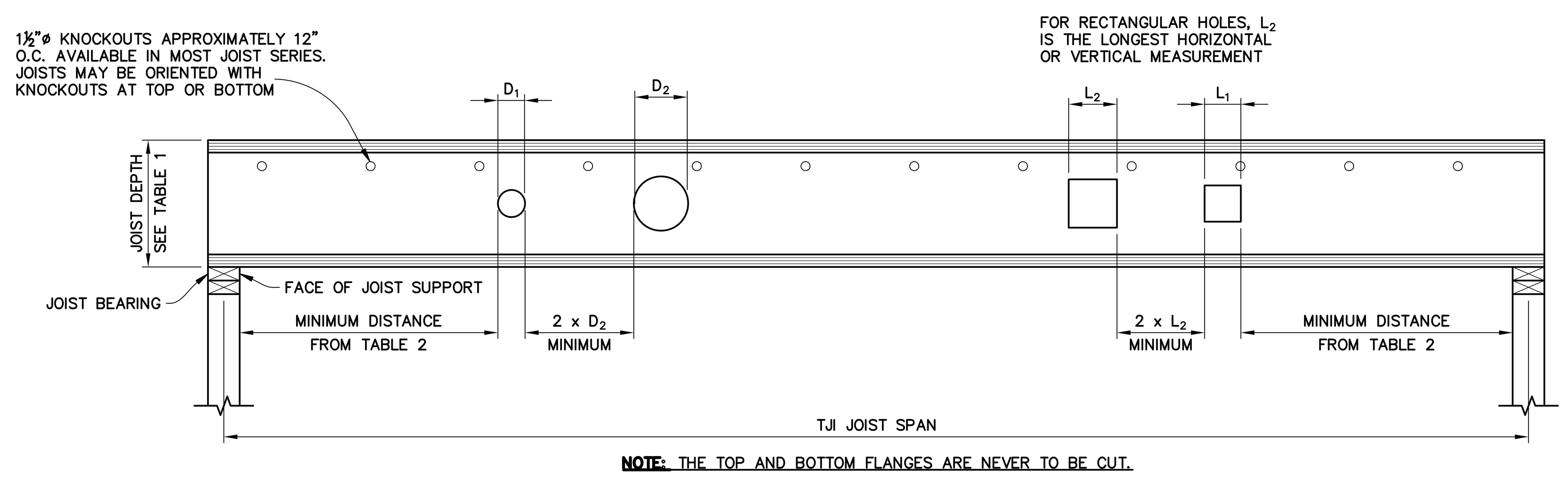


NOTES:

1. WEB STIFFENERS MUST BE FIELD INSTALLED AT BEARING POINTS & CONCENTRATED LOADS AS SHOWN ON THE DRAWINGS
2. GAP MUST BE AT TOP AT ALL BEARING CONDITIONS. IN THE CASE OF CONCENTRATED LOADS, THE GAP MUST BE AT THE BOTTOM
3. WEB STIFFENERS SHALL BE FURNISHED BY JOIST MANUFACTURER

TYP. JOIST WEB STIFFENER ATTACHMENT — 4 / S1.2

SCALE: NO SCALE 1309TYP06301



NOTE: THE TOP AND BOTTOM FLANGES ARE NEVER TO BE CUT.

TABLE 1 - HOLE SIZE FACTORS AND LOCATIONS

ROUND HOLE SIZE	RECTANGULAR HOLE SIZE(1)	JOIST DEPTH									
		11⅞"	14"	16"	18"	20"	22"	24"	26"	28"	30" OR 32"
2"	1¼"	A	A	A	4"	4"	4"	4"	4"	4"	4"
4"	2¼"	B	B	A	A	A	1'-3"	1'-3"	1'-3"	1'-3"	4"
6"	3½"	E	C	C	B	B	A	A	A	A	1'-3"
8"	4¾"		E	D	C	C	B	B	B	A	A
10"	6"			E	E	D	C	V	B	B	B
12"	7"					E	D	D	C	C	B
14"	8¼"						E	E	D	D	C
16"	9½"							E	E	D	D
18"	10¾"									E	E
20"	12"										E

TABLE 2 - HOLE LOCATIONS

JOIST SPAN	HOLE FACTORS				
	A	B	C	D	E
14'	1'-3"	2'-0"	2'-6"	3'-9"	5'-0"
16'	1'-3"	2'-3"	3'-3"	4'-6"	5'-9"
18'	1'-6"	3'-0"	4'-3"	5'-6"	6'-9"
20'	1'-9"	3'-0"	4'-3"	5'-6"	7'-0"
22'	2'-0"	3'-0"	4'-3"	5'-9"	7'-3"
24'	2'-3"	3'-3"	4'-6"	5'-9"	7'-6"
26'	2'-3"	3'-9"	4'-9"	6'-0"	7'-9"
28'	2'-6"	4'-0"	5'-3"	6'-6"	8'-0"
30'	2'-9"	4'-3"	5'-9"	7'-0"	8'-6"
32'	3'-0"	4'-"	6'-0"	7'-6"	9'-3"
34'	3'-0"	5'-0"	6'-6"	8'-0"	9'-9"
36'	3'-3"	5'-0"	6'-9"	8'-6"	10'-3"

TO DETERMINE HOLE LOCATION

1. DETERMINE THE JOIST DEPTH AND DESIRED HOLE SIZE AND FIND HOLE FACTOR (LETTER) OR HOLE LOCATION (DISTANCE) IN TABLE 1. IF THE TABLE REPORTS A HOLE FACTOR, PROVEED TO STEP 2
2. REFER TO TABLE 2. THE DIMENSION SHOWN WHERE THE JOIST SPAN AND HOLE FACTOR FROM TABLE 1 INTERSECT IS THE REQUIRED MINIMUM DISTANCE FROM THE NEAREST EDGE OF HOLE TO INSIDE FACE OF SUPPORT

SPECIAL CONSIDERATIONS

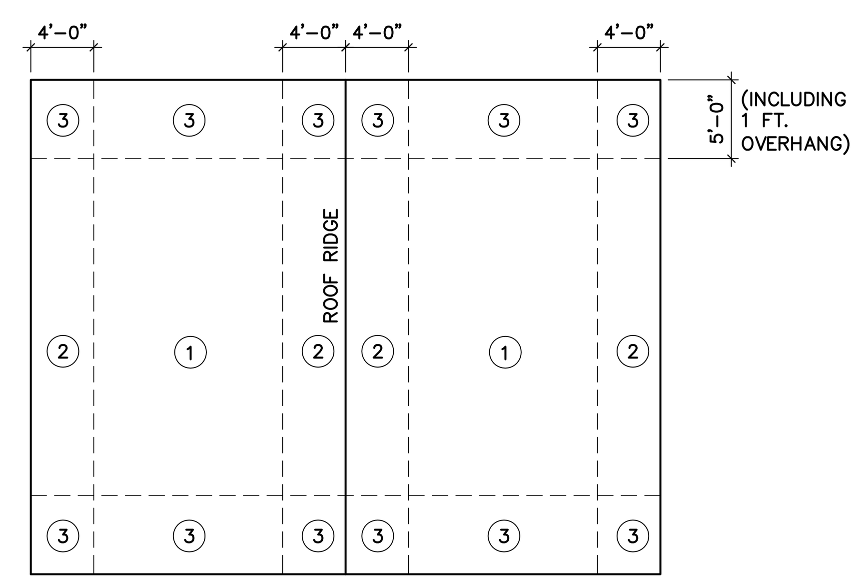
- TABLES ARE BASE ON:
  - SIMPLE SPANS
  - MORE RESTRICTIVE OF EITHER UNIFORM LOADS OR CODE-REQUIRED CONCENTRATED LOADS (2,000lbs OVER 2½ FEET SQUARE AND OVER TWO JOISTS) WITH 25psf DEAD AND 20psf PARTITION LOAD
- MULTIPLE HOLES REQUIRE SPACING 2 TIMES THE LENGTH OF THE LARGEST HOLE
- FOR JOISTS CONTINUOUS OVER SUPPORT, ADD 1" TO DISTANCE SHOWN IN TABLE FOR EACH FOOT OF JOIST SPAN
- HOLES MAY BE LOCATED VERTICALLY ANYWHERE IN THE WEB. LEAVE ⅛" MINIMUM AT TOP AND BOTTOM OF HOLE
- DO NOT CUT HOLES IN CANTILEVER AREA WITHOUT CONSULTING STRUCTURAL ENGINEER

iLEVEL™ TRUS-JOIST® TJI JOIST HOLE SCHEDULE — 5 / S1.2

SCALE: NO SCALE 1309TYP06192

WIND REGION	NAILS	PANEL LOCATION	ROOF FASTENING ZONE 1	ROOF FASTENING ZONE 2	ROOF FASTENING ZONE 3
			FASTENING SCHEDULE (INCHES ON CENTER)		
≥100mph BASIC WIND SPEED	10d COMMON 2⅜"	PANEL EDGES[1]	6	6	4
		PANEL FIELD	8	6	6

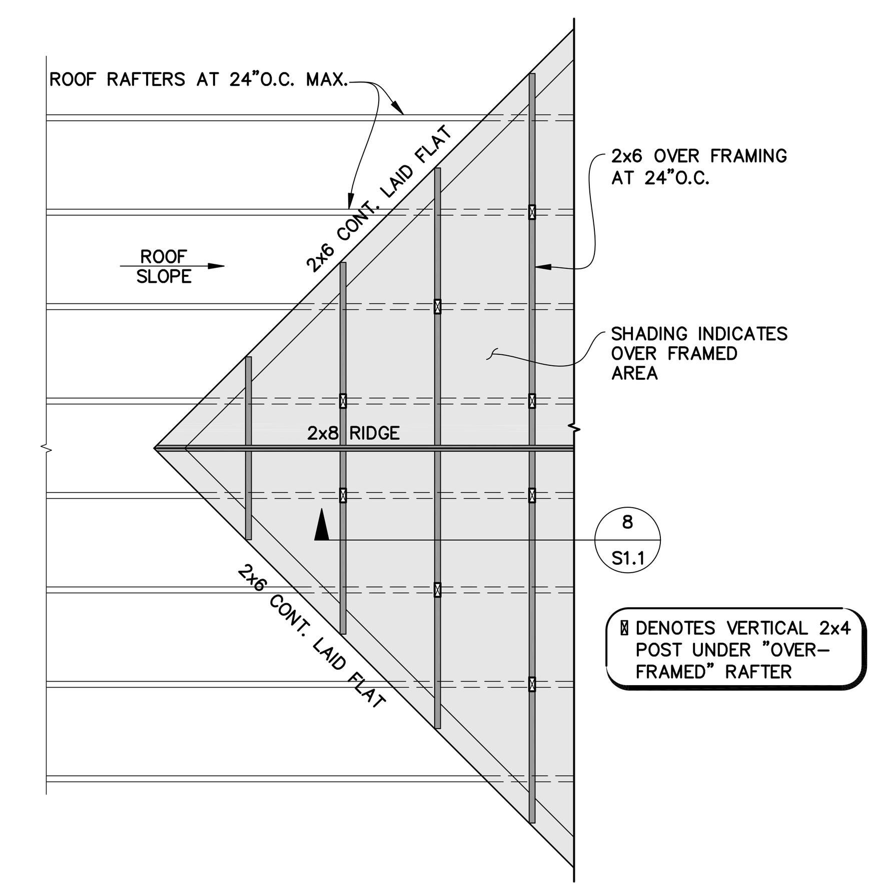
[1] EDGE SPACING ALSO APPLIES OVER ROOF FRAMING AT GABLE-END WALLS



ROOF FASTENING ZONES

2006 IRC TABLE 602.3(1) (MODIFIED) — 6 / S1.2

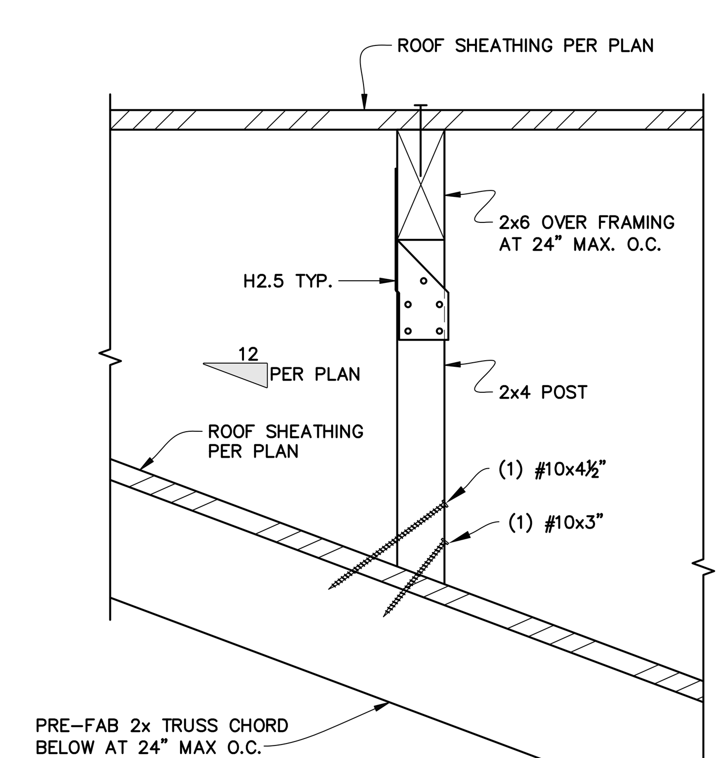
SCALE: NO SCALE 1309TYP06506



SEE ARCHITECTURAL DRAWINGS FOR ALL VENTILATION OF ATTIC AND OTHER "CLOSED-IN" SPACES

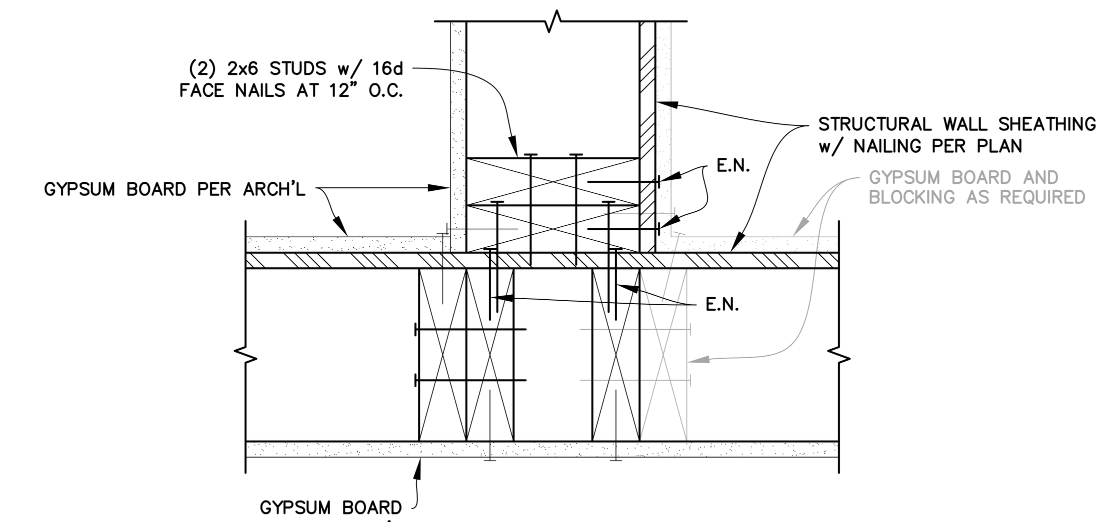
TYP. OVERFRAMING LAYOUT IN PLAN VIEW — 7 / S1.2

SCALE: ⅜" = 1'-0" 1309TYP06514



TYP. OVER FRAMING SECTION — 8 / S1.2

SCALE: 3" = 1'-0" 1309TYP06515



TYPICAL 2x6 WALL FRAMING "T" INTERSECTION DETAIL — 9 / S1.2

SCALE: 3" = 1'-0" 1309TYP06106A

NOTE:
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KNE
KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST., #425 RENO, NV 89501
PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:

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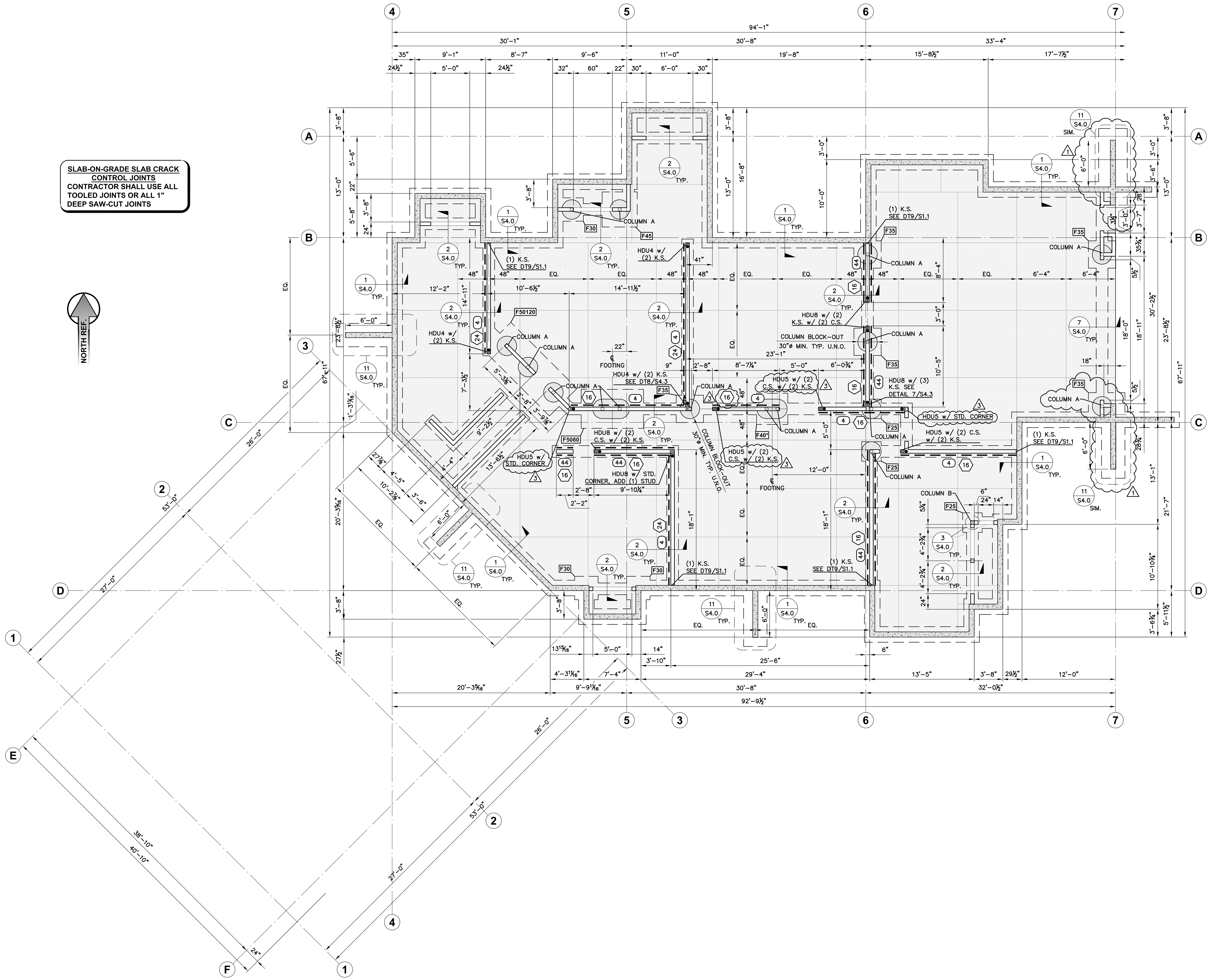
DESIGNED:	TRH/KNE
CHECKED:	KNE
KNE JOB NO.	1309

SHEET TITLE: TYPICAL DETAILS

SHEET NO. **S1.2** OF 15 SHEETS



BASEMENT FOOTING AND SHEAR WALL PLAN
SCALE: 3/16" = 1'-0"

1
S2.0

4007SPLANS

COLUMN SCHEDULE

DESIGNATION	MATERIAL
COLUMN A	HSS5x5x3/16
COLUMN B	6x6 D.F. #1

COORDINATE EXACT STEEL COLUMN ℄ LOCATIONS WITH STEEL FRAMING AND DETAILS OVERHEAD. SUBMIT SHOP DRAWINGS.

BASEMENT FRAMING NOTES

1. ALL GRIDLINES SHOWN ARE TO THE FACE OF STUD, TYP., U.N.O.
2. ALL EXTERIOR WALLS ARE 8" THICK CONC. WALLS w/ #5 REBAR AT 16" O.C. BOTH WAYS. TYP. U.N.O. (PROVIDE 12" THICK (8" + 4" LEDGE) CONC WALLS w/ #5 REBAR AT 16" O.C. B.W. AT WALL THAT WILL CARRY STONE VENEER ABOVE.
3. PROVIDE 4x8 D.F. P.T. SILL PLATES w/ 5/8"øx8" SIMPSON TITEN-HD CONC. SCREW ANCHOR (THD63800H) w/ 3"SQ. x1/4" THICK ℄ WASHERS AT 32" O.C. (MAX.), TYP. U.N.O.
4. INTERIOR STRUCTURAL BEARING WALLS SHALL CONSIST OF 2x6 D.F.#2 WOOD STUDS AT 16" O.C. (MAX.), TYP., U.N.O.
5. INDICATES SHEARWALL LOCATIONS WITH SHEATHING AND NAILING AS SHOWN ON THIS SHEET.
6. ● INDICATES HOLDOWN LOCATIONS, AS SHOWN AND DESIGNATED ON THE PLANS. SEE PLANS OR HOLDOWN SCHEDULE FOR REQUIRED POST ATTACHMENT.
7. COORDINATE DIMENSIONS, OPENING AND INFORMATION WITH THE ARCHITECTURAL DRAWINGS AS REQUIRED. REPORT AND DISCREPANCIES TO STRUCTURAL ENGINEER.

TYPICAL FOOTING SCHEDULE

FOOTING DESIGNATION	PLAN DIMENSION	THICKNESS	REINFORCING	TOTAL ALLOW. DOWNWARD LOAD
F20	2'-0" SQ.	16"	(3) #5 B.W. BOT.	10,000#
F25	2'-6" SQ.	16"	(3) #5 B.W. BOT.	15,625#
F30	3'-0" SQ.	16"	(4) #5 B.W. BOT.	22,500#
F35	3'-6" SQ.	16"	(4) #5 B.W. BOT.	30,625#
F40	4'-0" SQ.	16"	(5) #5 B.W. BOT.	40,000#
F40*	4'-0" SQ.	16"	(5) #5 B.W. T&B	40,000#
F45	4'-6" SQ.	16"	(6) #5 B.W. T&B	50,625#
F5080	5'-0"x8'-0"	16"	(6) #5 LONG. T&B (9) #5 TRAN. T&B	100,000#
F50120	5'-0"x12'-0"	16"	(6) #5 LONG. T&B (12) #5 TRAN. T&B	150,000#

- NOTE: ALL LOADS ARE ALLOWABLE STRESS DESIGN (UNFACTORED)
- SEE 10/S4.0 FOR TYPICAL INFORMATION.
- SOIL BEARING PRESSURES CALCULATED AT 2,500psf

ANCHOR BOLT SCHEDULE

SYMBOL	ANCHOR BOLT REQUIREMENTS
12	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **12" O.C.** WITH SIMPSON BP5/8-3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.
16	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **16" O.C.** WITH SIMPSON BP5/8-3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.
24	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **24" O.C.** WITH SIMPSON BP5/8-3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.
32	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **32" O.C.** WITH SIMPSON BP5/8-3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.

WALL TO FOUNDATION HOLDOWN SCHEDULE

DESIGNATION	HOLDOWN TYPE	ANCHOR TYPE	MIN. HOLDOWN POST THICK.	ALLOWABLE TENSION LOAD
HDU2	HDU2-SDS2.5	THD62800	(2) 2x6	3,075lbs
HDU4	HDU4-SDS2.5	THD62800	(2) 2x6	4,420lbs
HDU5	HDU5-SDS2.5	THD62800	(2) 2x6	5,175lbs
HDU8	HDU8-SDS2.5	THD871000	(3) 2x6	7,870lbs
HDU11	HDU11-SDS2.5	1"øx16" ASTM A307 ALL-THREAD ROD w/ SIMPSON SEX-XP EPOXY	(4) 2x6 OR 6x6	9,535lbs
			(5) 2x6 OR 6x8	10,100lbs
HDU14	HDU14-SDS2.5	1"øx16" ASTM A307 ALL-THREAD ROD w/ SIMPSON SEX-XP EPOXY	6x6	14,925lbs
HDQ8	HDQ8-SDS3	1"øx16" ASTM A307 ALL-THREAD ROD w/ SIMPSON SEX-XP EPOXY	(3) 2x6	9,230lbs
HHDQ11	HHDQ11-SDS2.5	1"øx16" ASTM A307 ALL-THREAD ROD w/ SIMPSON SEX-XP EPOXY	6x6	11,810lbs
HHDQ14	HHDQ14-SDS2.5	1"øx16" ASTM A307 ALL-THREAD ROD w/ SIMPSON SEX-XP EPOXY	6x6	13,710lbs

- ALL HOLDOWNS SHALL BE PRODUCED BY SIMPSON STRONG-TIE® ONLY. ANY PRODUCER OR PRODUCT SUBSTITUTIONS MUST BE APPROVED IN WRITING BY KIRK N. ELLIS & ASSOCIATES. PRIOR TO INSTALLATION.
- INSTALL ALL HOLDOWNS AND ANCHORS PER MANUFACTURER'S REQUIREMENTS AND SPECIFICATIONS
- SIMPSON SDS SCREW LENGTH ASSUMES DIRECT CONNECTION WITH POST SUPPORT. WHERE REQUIRED, INCREASE THE LENGTH OF SDS SCREWS THE THICKNESS OF SHEATHING MATERIAL AT INTERSECTING WALLS. (i.e. USE SDS25300 IN LIEU OF SDS25212 SCREWS AT 1/2" THICK SHEATHING FOR A WALL WITH CONT. SHEATHING INTERSECTING THE WALL REQUIRING THE HOLDOWN)
- HOLDOWNS SPECIFIED AT STEEL BEAMS SHALL USE SAME DIAMETER ANCHOR AS INDICATED ON THIS SCHEDULE, WELDED TO STEEL BEAM BELOW. SEE DETAIL 10/S4.2.
- USE ASTM A307 ALL-THREAD ROD WITH SIMPSON SET-XP EPOXY. PROVIDE MINIMUM 12" EMBEDMENT ON ALL ANCHORS AND INSTALL PER MANUFACTURER'S INSTRUCTIONS. TYP. U.N.O.

2

SHEATHING & SHEAR NAILING SCHEDULE FOR SHEAR WALLS

SYMBOL	PLYWOOD AND NAILING REQUIREMENT
6	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **6" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE 2" NOMINAL OR THICKER AT ABUTTING PANEL EDGES.
4	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **4" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE 2" NOMINAL OR THICKER AT ABUTTING PANEL EDGES.
3	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **3" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
2	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **2" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
44	APPLY 1/2" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **4" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
33	APPLY 1/2" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **3" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
22	APPLY 1/2" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2 1/4" NAILS AT **2" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.

WHERE ALL SHEARWALLS INTERSECT AT A CORNER, PROVIDE THE TIGHTER OF THE TWO NAILING SCHEDULES TO BOTH SIDES OF THE CORNER, TYP.

NOTE:
CONTRACTOR SHALL COORDINATE ALL DIMENSIONS, CONDITIONS AND DETAILS WITH THE ARCHITECTURAL DRAWINGS. REPORT ANY DISCREPANCIES TO STRUCTURAL ENGINEER. TYP. U.N.O. COORD. ALL AS REQ'D.

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PROFESSIONAL ENGINEER - STATE OF NEVADA
KIRK N. ELLIS
Exp: 12/31/10
CIVIL / STRUCTURAL
No. 4398
5/27/2009

KNE
KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST., #425 RENO, NV 89501
PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:
1. 08/31/2009 ARCH'L COORD.
2. 08/31/2009 BUILDING DEPT.
3. 09/10/2009 BUILDING DEPT.

DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
DESIGNED: TRH/KNE
CHECKED: KNE
KNE JOB NO. 1309

SHEET TITLE: STRUCTURAL BASEMENT FOUNDATION PLAN

SHEET NO. S2.0
OF 15 SHEETS

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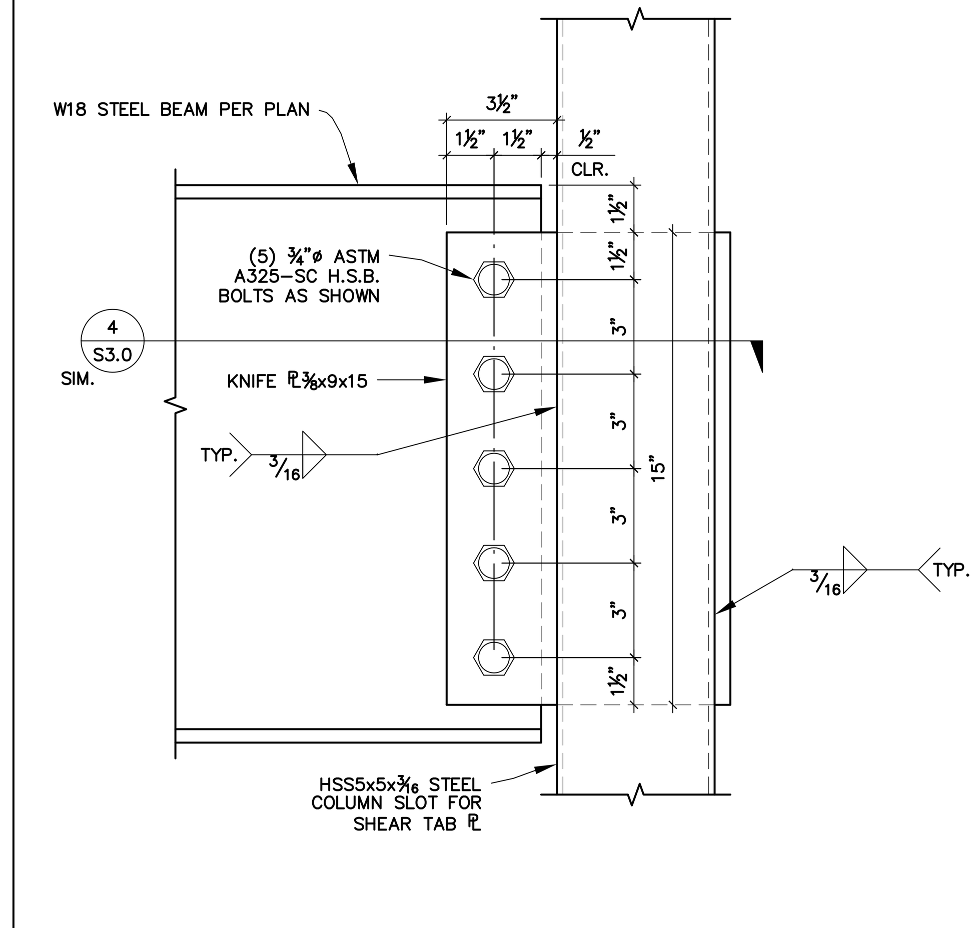
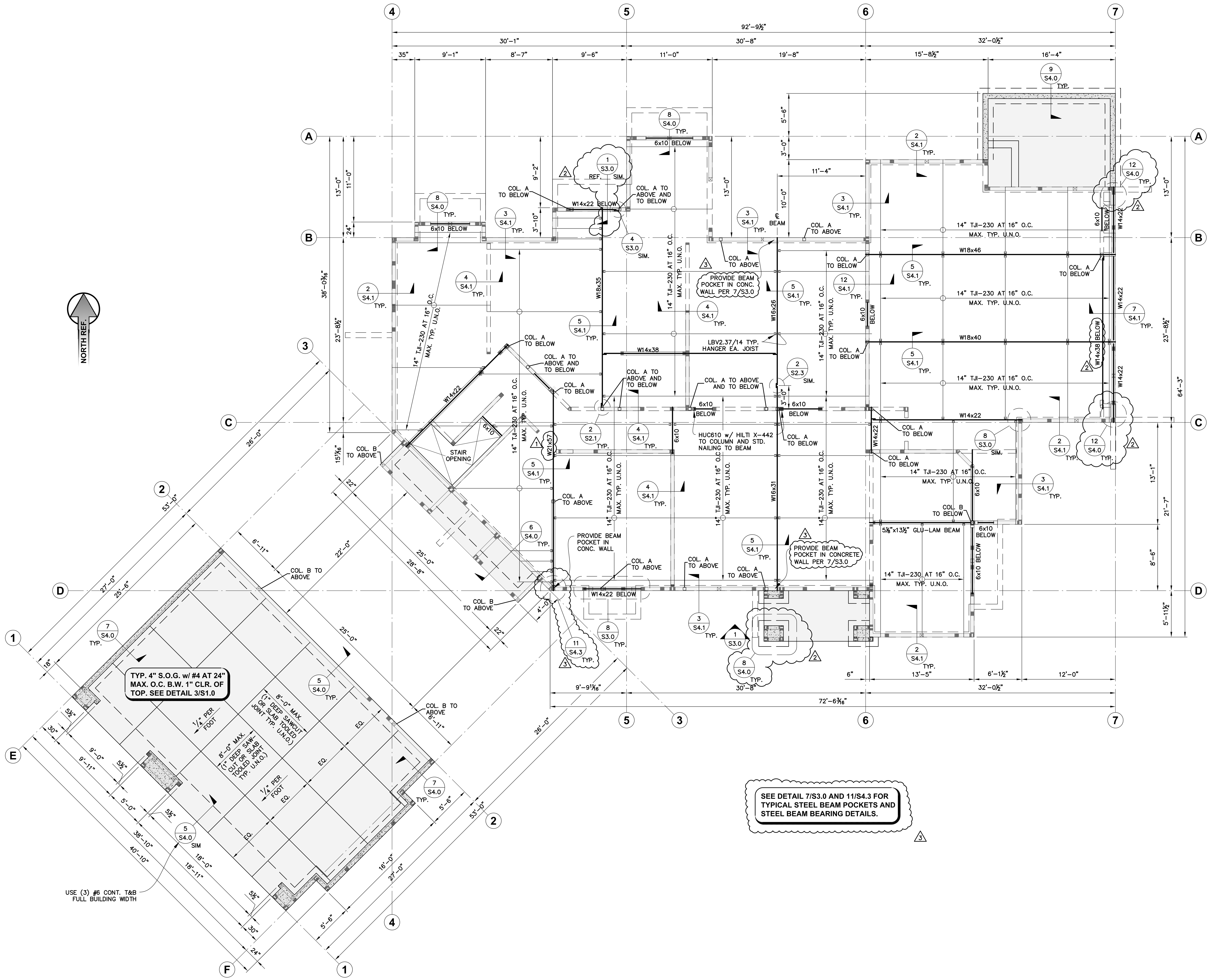


TYPICAL W18 STEEL BEAM TO HSS COL. DETAIL 2 / S2.1

SCALE: 3" = 1'–0" 1309DT48



GARAGE AND FIRST FLOOR FRAMING PLAN 1 / S2.1

SCALE: 3/16" = 1'–0" 4007SPLANS

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COLUMN SCHEDULE

DESIGNATION	MATERIAL
COLUMN A	HSS5x5x3/16
COLUMN B	6x6 D.F. #1

COORDINATE EXACT STEEL COLUMN ℄ LOCATIONS WITH STEEL FRAMING AND DETAILS OVERHEAD. SUBMIT SHOP DRAWINGS.

FIRST FLOOR FRAMING NOTES

1. ALL GRIDLINES SHOWN ARE TO THE FACE OF STUD TYP., U.N.O.
2. ALL EXTERIOR WALLS BELOW ARE 8" THICK CONC. WALLS w/ #5 REBAR AT 16" O.C. BOTH WAYS. TYP. U.N.O.
3. PROVIDE 4x8 D.F. P.T. SILL PLATES w/ ⅝"øx8" SIMPSON TITEN–HD CONC. SCREW ANCHOR (THD62800H) w/ SIMPSON BP⅝–3 ℞ WASHERS AT 32" O.C. (MAX.), TYP. U.N.O.
4. EXTERIOR WALLS ABOVE AND INTERIOR STRUCTURAL BEARING WALLS SHALL CONSIST OF 2x6 D.F.#2 WOOD STUDS AT 16" O.C. (MAX.), TYP., U.N.O.
5. ALL EXTERIOR WALLS SHALL BE SHEATHED w/ ½" O.S.B. WOOD SHEATHING, NAILED WITH 10dx2¼" NAILS AT MIN. 6" O.C. EDGE, 10" O.C. FIELD TYP., U.N.O.
6. FLOOR JOIST SHALL BE iLEVEL (BY WEYERHAEUSER) TJI–230 14" DEEP WITH A MAXIMUM SPAN OF 15'–6" BEARING EDGE TO BEARING EDGE. DOUBLE JOIST REQUIRED AT PARALLEL WALLS ABOVE (BOTH STRUCTURAL AND NON–STRUCTURAL WALLS). PROVIDE TYPICAL JOIST HANGER LBV2.37/14 TYP. U.N.O. MAXIMUM SPACING IS 16" O.C. TYP. U.N.O.
7. USE 1⅛" APA RATED (48/42) STURD–I–FLOOR SHEATHING. GLUE AND NAIL w/ 10dx3" SCREW NAILS AT 6" O.C. EDGE, AND 8" O.C. FIELD, TYP. U.N.O. OF FLOOR SHEATHING =–0'–1½". PROVIDE 1½" THICK LIGHT–WEIGHT CONCRETE, T.O.F.F. =+0'–0"
8. COORDINATE DIMENSIONS, OPENING AND INFORMATION WITH THE ARCHITECTURAL DRAWINGS AS REQUIRED. REPORT AND DISCREPANCIES TO STRUCTURAL ENGINEER.

NOTE:
CONTRACTOR SHALL COORDINATE ALL DIMENSIONS, CONDITIONS AND DETAILS WITH THE ARCHITECTURAL DRAWINGS. REPORT ANY DISCREPANCIES TO STRUCTURAL ENGINEER. TYP. U.N.O. COORD. ALL AS REQ'D.

ISSUED FOR BUILDING DEPARTMENT PLAN CHECK
May 27, 2009

NOTE:
TYPICAL DETAILS ARE NOT USUALLY CUT OR REFERENCED. THEY ARE INTENDED TO APPLY IN ALL SUCH SIMILAR CONDITIONS, WHETHER REFERENCED OR NOT.

PROFESSIONAL ENGINEER – STATE OF NEVADA – KIRK N. ELLIS – No. 4398 – STRUCTURAL – 5/27/2009

KNE
KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST., #425 RENO, NV 89501
PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

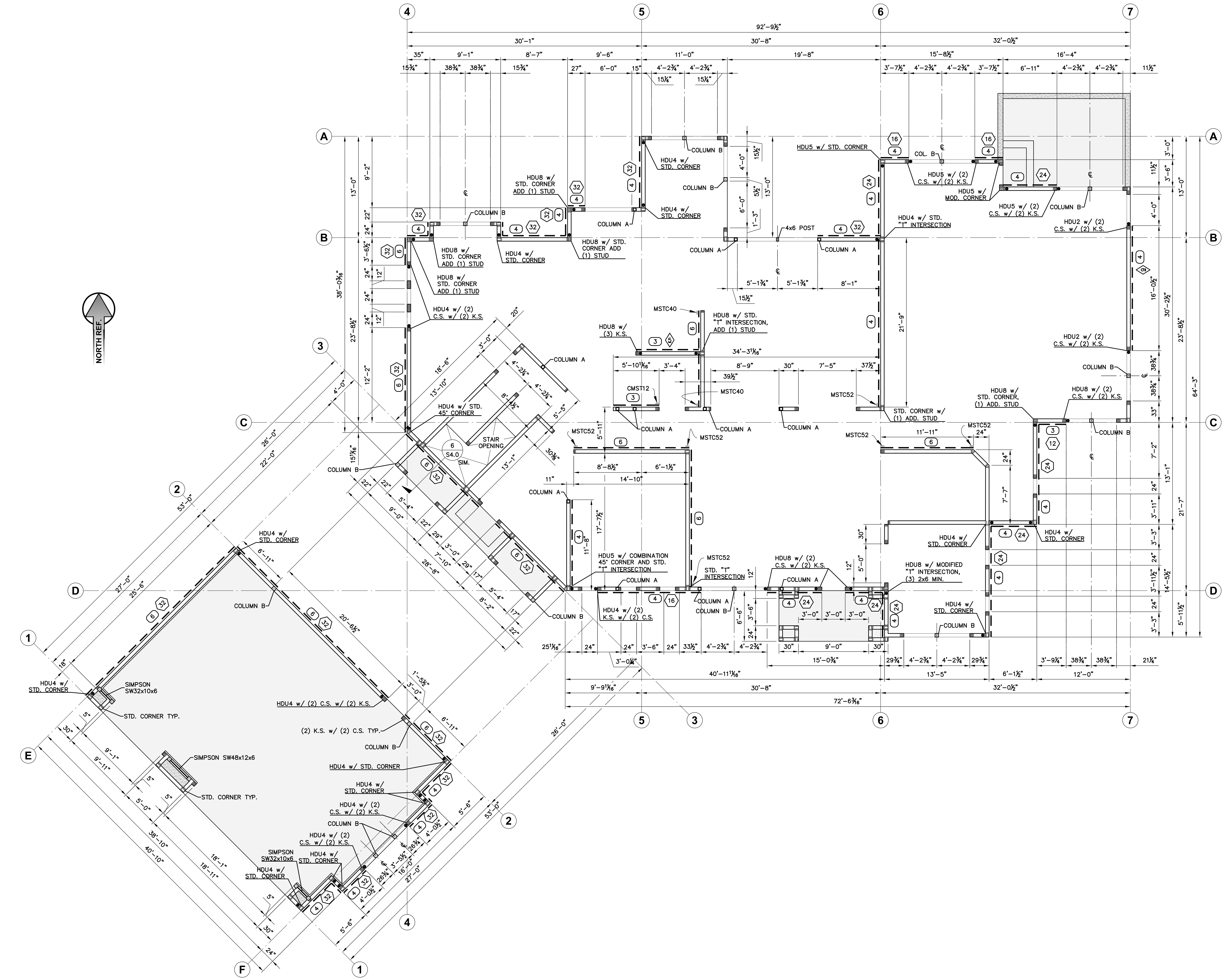
Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:
1. 08/31/2009 ARCH'L COORD.
2. 08/31/2009 BUILDING DEPT.
3. 09/10/2009 BUILDING DEPT.

DATE:	05/27/2009
SCALE:	AS SHOWN
DRAWN:	BLW
DESIGNED:	TRH/KNE
CHECKED:	KNE
JOB NO.:	1309

SHEET TITLE: STRUCTURAL FIRST FLOOR FRAMING DETAIL

SHEET NO.
S2.1
OF 15 SHEETS



FIRST FLOOR SHEAR WALL PLAN
SCALE: 3/16" = 1'-0"

1/S2.2

4007SPLANS

COLUMN SCHEDULE

DESIGNATION	MATERIAL
COLUMN A	HSS5x5x3/16
COLUMN B	6x6 D.F. #1

COORDINATE EXACT STEEL COLUMN ℄ LOCATIONS WITH STEEL FRAMING AND DETAILS OVERHEAD. SUBMIT SHOP DRAWINGS.

FIRST FLOOR WALL FRAMING NOTES

1. ALL GRIDLINES SHOWN ARE TO THE FACE OF STUD, TYP., U.N.O.
2. EXTERIOR WALLS AND INTERIOR STRUCTURAL BEARING WALLS SHALL CONSIST OF 2x6 D.F.#2 WOOD STUDS AT 16" O.C. (MAX.), TYP., U.N.O.
3. ALL EXTERIOR WALLS SHALL BE SHEATHED w/ 1/2" O.S.B. WOOD SHEATHING, NAILED WITH 10dx2¼" NAILS AT MIN. 6" O.C. EDGE, 10" O.C. FIELD TYP., U.N.O.
4. INDICATES SHEARWALL LOCATION WITH SHEATHING AND NAILING ON SIDE(S) AS SHOWN ON THIS SHEET.
5. ● INDICATES HOLDOWN/STRAP LOCATIONS, AS SHOWN AND DESIGNATED ON THE PLANS. SEE PLANS OR HOLDOWN/STRAP SCHEDULE FOR REQUIRED POST ATTACHMENT.
6. COORDINATE DIMENSIONS, OPENING AND INFORMATION WITH THE ARCHITECTURAL DRAWINGS AS REQUIRED. REPORT AND DISCREPANCIES TO STRUCTURAL ENGINEER.

ANCHOR BOLT SCHEDULE

SYMBOL	ANCHOR BOLT REQUIREMENTS
12	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **12" O.C.** WITH SIMPSON BP5/8–3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.
16	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **16" O.C.** WITH SIMPSON BP5/8–3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.
24	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **24" O.C.** WITH SIMPSON BP5/8–3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.
32	USE 5/8"øx8" SIMPSON CONCRETE SCREW ANCHOR (THD62800H) BOLTS AT **32" O.C.** WITH SIMPSON BP5/8–3 PLATE WASHERS AND 2" NOMINAL SILL PLATE.

WALL TO FOUNDATION HOLDOWN SCHEDULE

DESIGNATION	HOLDOWN TYPE	ANCHOR TYPE	MIN. HOLDOWN POST THICK.	ALLOWABLE TENSION LOAD
HDU2	HDU2–SDS2.5	THD62800	(2) 2x6	3,075lbs
HDU4	HDU4–SDS2.5	THD62800	(2) 2x6	4,420lbs
HDU5	HDU5–SDS2.5	THD62800	(2) 2x6	5,175lbs
HDU8	HDU8–SDS2.5	THD871000	(3) 2x6	7,870lbs
HDU11	HDU11–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	(4) 2x6 OR 6x6	9,535lbs
			(5) 2x6 OR 6x8	10,100lbs
HDU14	HDU14–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	6x6	14,925lbs
HDQ8	HDQ8–SDS3	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	(3) 2x6	9,230lbs
HHDQ11	HHDQ11–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	6x6	11,810lbs
HHDQ14	HHDQ14–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	6x6	13,710lbs

- ALL HOLDOWNS SHALL BE PRODUCED BY SIMPSON STRONG–TIE® ONLY. ANY PRODUCER OR PRODUCT SUBSTITUTIONS MUST BE APPROVED IN WRITING BY KIRK N. ELLIS & ASSOCIATES. PRIOR TO INSTALLATION.
- INSTALL ALL HOLDOWNS AND ANCHORS PER MANUFACTURER'S REQUIREMENTS AND SPECIFICATIONS
- SIMPSON SDS SCREW LENGTH ASSUMES DIRECT CONNECTION WITH POST SUPPORT. WHERE REQUIRED, INCREASE THE LENGTH OF SDS SCREWS THE THICKNESS OF SHEATHING MATERIAL AT INTERSECTING WALLS. (i.e. USE SDS25300 IN LIEU OF SDS25212 SCREWS AT 1/2" THICK SHEATHING FOR A WALL WITH CONT. SHEATHING INTERSECTING THE WALL REQUIRING THE HOLDOWN)
- HOLDOWNS SPECIFIED AT STEEL BEAMS SHALL USE SAME DIAMETER ANCHOR AS INDICATED ON THIS SCHEDULE, WELDED TO STEEL BEAM BELOW. SEE DETAIL 10/S4.2.
- 2 USE ASTM A307 ALL–THREAD ROD WITH SIMPSON SET–XP EPOXY. PROVIDE MINIMUM 12" EMBEDMENT ON ALL ANCHORS AND INSTALL PER MANUFACTURER'S INSTRUCTIONS. TYP. U.N.O.

WALL TO WALL STRAP SCHEDULE

STRAP TYPE	FLOOR-TO-FLOOR CLEAR SPAN	REQ'D FASTENERS (TOTAL)	ALLOWABLE TENSION LOAD
MSTA49	≤18"	(26) 10d	2,020lbs
MSTC40	18"	(28) 16d SINKERS	2,290lbs
	16"	(36) 16d SINKERS	2,945lbs
	0"	(52) 16d SINKERS	4,265lbs
MSTC52	18"	(44) 16d SINKERS	3,595lbs
	16"	(48) 16d SINKERS	3,925lbs
	0"	(62) 16d SINKERS	4,745lbs
MSTC66	18"	(64) 16d SINKERS	5,460lbs
	16"	(68) 16d SINKERS	5,800lbs
	0"	(74) 16d SINKERS	5,800lbs
CMST14	≤18"	(66) 16d	6,490lbs
CMST12	≤18"	(84) 16d	9,215lbs

- ALL STRAP TIE HOLDOWNS SHALL BE PRODUCED AND MANUFACTURED BY SIMPSON STRONG–TIE ONLY. ANY PRODUCT OR MANUFACTURER SUBSTITUTIONS MUST BE APPROVED IN WRITING BY KIRK N. ELLIS & ASSOCIATES PRIOR TO INSTALLATION.
- INSTALL ALL STRAPS PER MANUFACTURER'S INSTRUCTIONS.
- NAILS ARE NOT REQUIRED IN FLOOR–TO–FLOOR CLEAR SPAN AND ARE NOT COUNTED IN THE REQUIRED TOTAL FASTENERS
- ALL STRAPS REQUIRE (2) 2x STUDS AT CENTERLINE OF STRAP.

SHEATHING & SHEAR NAILING SCHEDULE FOR SHEAR WALLS

SYMBOL	PLYWOOD AND NAILING REQUIREMENT
6	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **6" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE 2" NOMINAL OR THICKER AT ABUTTING PANEL EDGES.
4	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **4" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE 2" NOMINAL OR THICKER AT ABUTTING PANEL EDGES.
3	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **3" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
2	APPLY 1/2" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **2" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
44	APPLY 1/2" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2¼" NAILS AT **4" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
33	APPLY 1/2" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2¼" NAILS AT **3" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
22	APPLY 1/2" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2¼" NAILS AT **2" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.

WHERE ALL SHEARWALLS INTERSECT AT A CORNER, PROVIDE THE TIGHTER OF THE TWO NAILING SCHEDULES TO BOTH SIDES OF THE CORNER, TYP.

POWDER DRIVEN PIN ANCHORAGE SPACING

SYMBOL	REQUIREMENT
9	(2) ROWS HILTI X–U72 PINS AT 18" O.C. MAX. STAGGERED TO NET 9" O.C. MAX. IN BOTTOM LAYER OF DOUBLE 2x6 BOTTOM ℙ.
6	(2) ROWS HILTI X–U72 PINS AT 12" O.C. MAX. STAGGERED TO NET 6" O.C. MAX. IN BOTTOM LAYER OF DOUBLE 2x6 BOTTOM ℙ.
5	(2) ROWS HILTI X–U72 PINS AT 10" O.C. MAX. STAGGERED TO NET 5" O.C. MAX. IN BOTTOM LAYER OF DOUBLE 2x6 BOTTOM ℙ.

NOTE:
CONTRACTOR SHALL COORDINATE ALL DIMENSIONS, CONDITIONS AND DETAILS WITH THE ARCHITECTURAL DRAWINGS. REPORT ANY DISCREPANCIES TO STRUCTURAL ENGINEER. TYP. U.N.O. COORD. ALL AS REQ'D.

ISSUED FOR BUILDING DEPARTMENT PLAN CHECK
May 27, 2009

NOTE:
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PROFESSIONAL ENGINEER - STATE OF NEVADA
KIRK N. ELLIS
Exp: 12/31/10
CIVIL / STRUCTURAL
No. 4398
5/27/2009

KNE
KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST., #425 RENO, NV 89501
PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

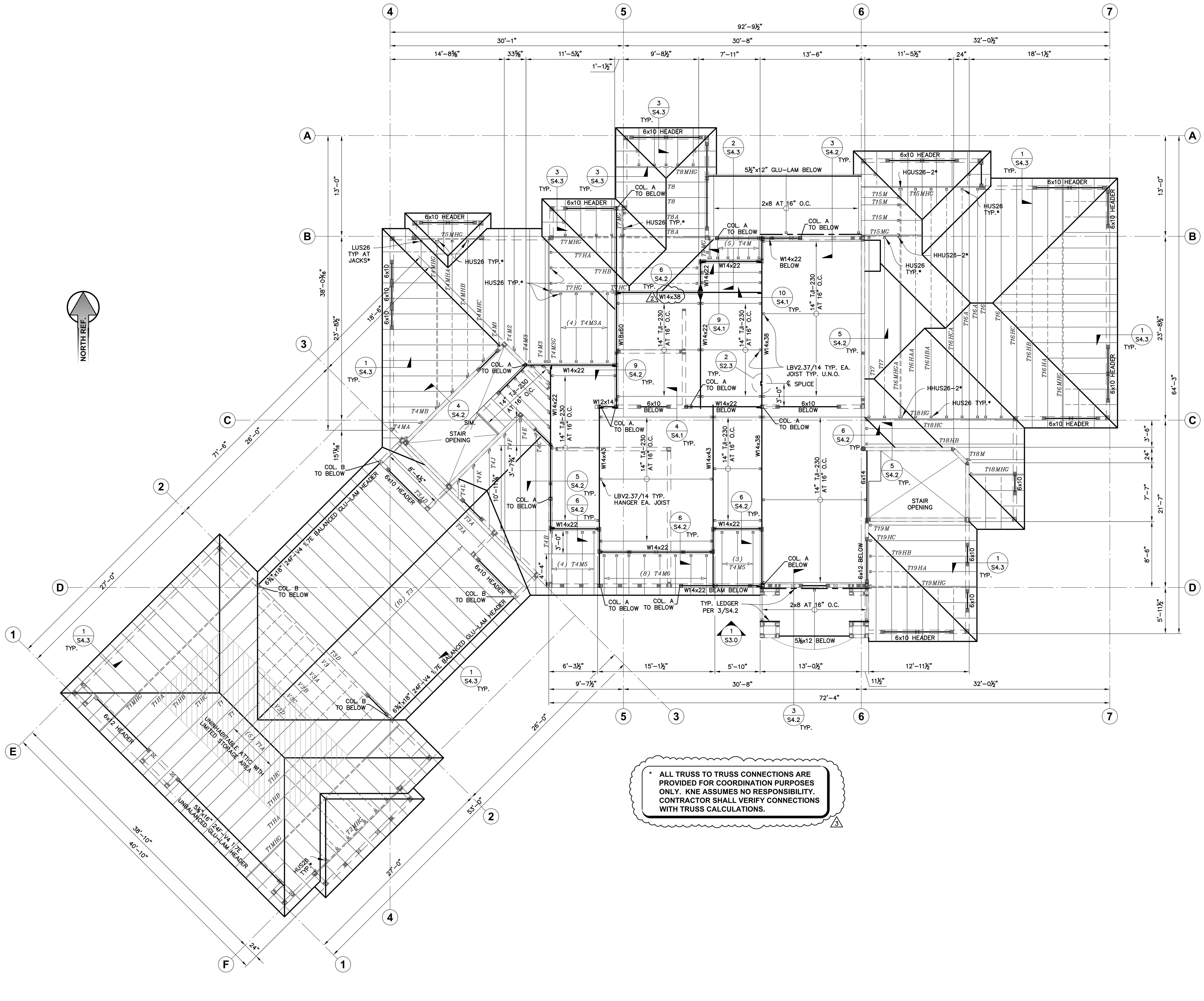
Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:
1 08/31/2009 ARCH'L COORD.
2 08/31/2009 BUILDING DEPT.
3 09/10/2009 BUILDING DEPT.

DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
DESIGNED: TRH/KNE
CHECKED: KNE
KNE JOB NO. 1309

SHEET TITLE: STRUCTURAL FIRST FLOOR WALL FRAMING PLAN

SHEET NO. S2.2
OF 15 SHEETS



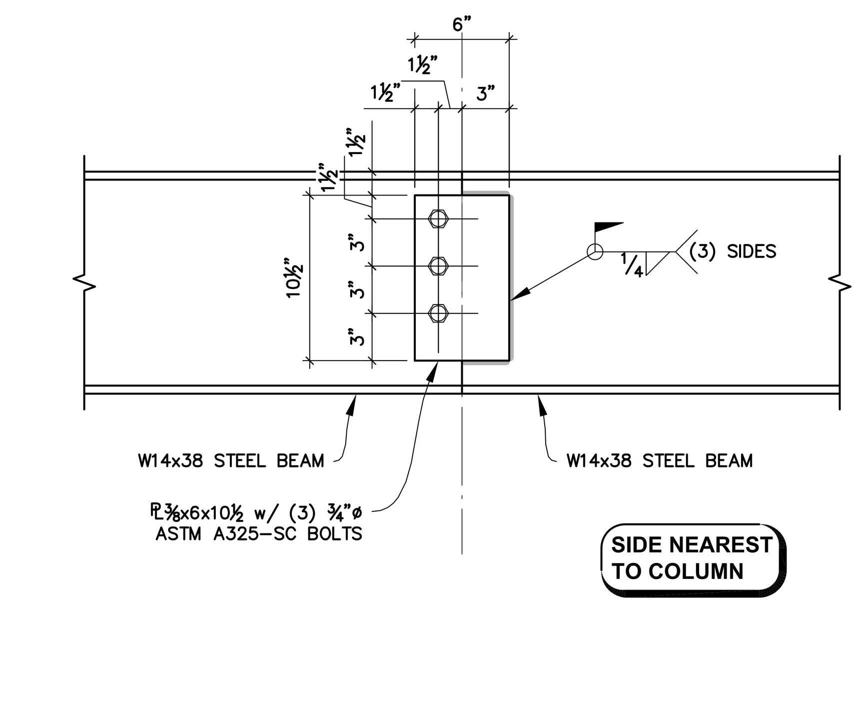
* ALL TRUSS TO TRUSS CONNECTIONS ARE PROVIDED FOR COORDINATION PURPOSES ONLY. KNE ASSUMES NO RESPONSIBILITY. CONTRACTOR SHALL VERIFY CONNECTIONS WITH TRUSS CALCULATIONS.

SECOND FLOOR AND LOWER ROOF FRAMING PLAN

SCALE: 3/16" = 1'-0"

4007SPLANS

1
S2.3



TYPICAL STEEL BEAM SPLICE DETAIL

SCALE: 1½" = 1'-0"

1309DT47

2
S2.3

TYPICAL TRUSS TO STRUCTURE HANGER SCHEDULE

LOCATION	HANGER
TRUSS TO WALL	HUS28
TRUSS TO WALL 45°L	HU28SKL45°
TRUSS TO WALL 45°R	HU28SKR45°
(2) PLY TRUSS TO WALL	HUS28–2
(3) PLY TRUSS TO WALL	HUS28–3
(4) PLY TRUSS TO WALL	HUS28–4

COLUMN SCHEDULE

DESIGNATION	MATERIAL
COLUMN A	HSS5x5x3/16
COLUMN B	6x6 D.F. #1

COORDINATE EXACT STEEL COLUMN ℄ LOCATIONS WITH STEEL FRAMING AND DETAILS OVERHEAD. SUBMIT SHOP DRAWINGS.

SECOND FLOOR AND ROOF FRAMING NOTES

1. ALL GRIDLINES SHOWN ARE TO THE FACE OF STUD TYP., U.N.O.
2. EXTERIOR WALLS ABOVE AND INTERIOR STRUCTURAL BEARING WALLS SHALL CONSIST OF 2x6 D.F.#2 WOOD STUDS AT 16" O.C. (MAX.), TYP., U.N.O.
3. ALL EXTERIOR WALLS SHALL BE SHEATHED w/ ½" O.S.B. WOOD SHEATHING, NAILED WITH 10dx2¼" NAILS AT MIN. 6" O.C. EDGE, 10" O.C. FIELD TYP., U.N.O.
4. FLOOR JOIST SHALL BE iLEVEL (BY WEYERHAEUSER) TJI–230 14"" DEEP WITH A MAXIMUM SPAN OF 19'–9" BEARING EDGE TO BEARING EDGE. DOUBLE JOIST REQUIRED AT PARALLEL WALLS ABOVE (BOTH STRUCTURAL AND NON–STRUCTURAL WALLS). PROVIDE TYPICAL JOIST HANGER MIT411.88 TYP. U.N.O. MAXIMUM SPACING IS 16" O.C. TYP. U.N.O.
5. USE 1⅛" APA RATED (48/42) STURD–I–FLOOR SHEATHING. GLUE AND NAIL w/ 10dx3" SCREW NAILS AT 6" O.C. EDGE, AND 10" O.C. FIELD, TYP. U.N.O. OF FLOOR SHEATHING =11'–5⅝"
6. USE ⅝" APA RATED (40/20) CDX EXP. 1 STRUCTURAL SHEATHING WITH 10d COMMON x2⅜" NAILS AT 6" O.C. EDGE AND 12" O.C. FIELD, TYP. U.N.O. IN ADDITION, PROVIDE NAILING AS INDICATED IN DETAIL X/SX.X, TIGHTER NAILING REQUIREMENTS SHALL ALWAYS CONTRAOL WHERE THERE IS A CONFLICT.
7. CONTRACTOR SHALL PROVIDE POSITIVE ATTIC VENTILATION PER THE 2006 I.B.C. AND APPLICABLE WASHOE COUNTY CODES.
8. COORDINATE DIMENSIONS, OPENING AND INFORMATION WITH THE ARCHITECTURAL DRAWINGS AS REQUIRED. REPORT AND DISCREPANCIES TO STRUCTURAL ENGINEER.

PLYWOOD EDGE NAILING AND BLOCKING INFORMATION

BLOCK ALL PLYWOOD EDGES WITH 3x FLAT BLOCKING AND NAIL WITH 10d COMMON NAILS AT 6" O.C. BOUNDARY AND EDGE, AND 10" O.C. FIELD.

BLOCK ALL PLYWOOD EDGES WITH 3x FLAT BLOCKING AND NAIL WITH 10d COMMON NAILS AT 4" O.C. BOUNDARY AND EDGE, AND 10" O.C. FIELD.

BLOCK ALL PLYWOOD EDGES WITH 3x FLAT BLOCKING AND NAIL WITH 10d COMMON NAILS AT 3½" O.C. BOUNDARY AND EDGE, AND 10" O.C. FIELD.

TRUSS DESIGN INFORMATION

	DEAD LOAD MAXIMUM	DEAD LOAD MINIMUM	SNOW LOAD	WIND UPLIFT (2)
TOP CHORD	22.0 psf	10.0 psf	20 psf	29.6 psf
BTM. CHORD	10.0 psf	7.0 psf	**10 psf	N/A

** DESIGN BOTTOM CHORD FOR 10 PSF NON–CONCURRENT LIVE LOAD AND 200LB MOVING POINT LOAD

1. TRUSS WEB CONFIGURATIONS SHOWN ARE REPRESENTATIVE ONLY. ACTUAL TRUSS CONFIGURATIONS TO BE DETERMINED BY TRUSS COMPANY. ALL TOP CHORDS SHALL BE 2x6 TYP. U.N.O.
2. NORMAL FORCE TO ROOF ELEMENTS AND COMPONENTS PER 2006 IBC SEC. 1620
3. ALL TRUSS TO TRUSS CONNECTIONS ARE PER THE TRUSS MANUFACTURER AND MUST BE GOOD FOR BOTH GRAVITY AND WIND UPLIFT LOADS.
4. ALL LOADS SHOWN ABOVE ARE PROJECTED. EXCEPT THE WIND LOAD.
5. PER WASHOE COUNTY LOCAL REQUIREMENTS, INCREASE THE SNOW LOADS AT THE EAVES BY A FACTOR OF 2.0

NOTE:
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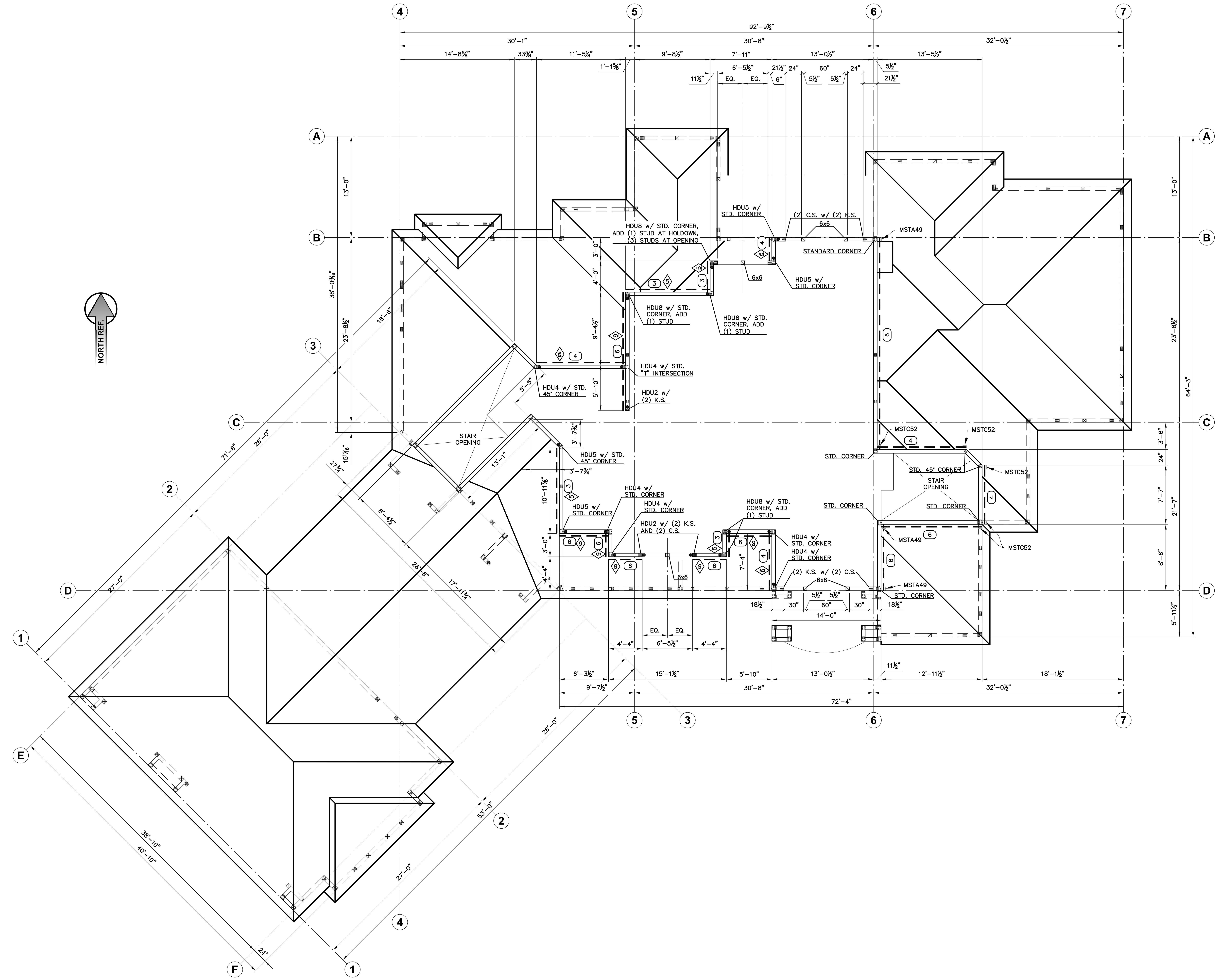
2. 09/10/2009 BUILDING DEPT.

DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
DESIGNED: TRH/KNE
CHECKED: KNE
KNE JOB NO. 1309

SHEET TITLE: STRUCTURAL SECOND FLOOR FRAMING PLAN

SHEET NO. S2.3 OF 15 SHEETS



SECOND FLOOR SHEAR WALL PLAN

SCALE: 3/16" = 1'-0"

1 / S2.4

4007SPLANS

SECOND FLOOR FRAMING NOTES

1. ALL GRIDLINES SHOWN ARE TO THE FACE OF STUD, TYP., U.N.O.
2. EXTERIOR WALLS AND INTERIOR STRUCTURAL BEARING WALLS SHALL CONSIST OF 2x6 D.F.#2 WOOD STUDS AT 16" O.C. (MAX.), TYP., U.N.O.
3. ALL EXTERIOR WALLS SHALL BE SHEATHED w/ ½" O.S.B. WOOD SHEATHING, NAILED WITH 10dx2¼" NAILS AT MIN. 6" O.C. EDGE, 10" O.C. FIELD TYP., U.N.O.
4. INDICATES SHEARWALL LOCATION WITH SHEATHING AND NAILING ON SIDE(S) AS SHOWN ON THIS SHEET.
5. ● INDICATES HOLDOWN/STRAP LOCATIONS, AS SHOWN AND DESIGNATED ON THE PLANS. SEE PLANS OR HOLDOWN/STRAP SCHEDULE FOR REQUIRED POST ATTACHMENT.
6. COORDINATE DIMENSIONS, OPENING AND INFORMATION WITH THE ARCHITECTURAL DRAWINGS AS REQUIRED. REPORT AND DISCREPANCIES TO STRUCTURAL ENGINEER.

WALL TO FOUNDATION HOLDOWN SCHEDULE

DESIGNATION	HOLDOWN TYPE	ANCHOR TYPE	MIN. HOLDOWN POST THICK.	ALLOWABLE TENSION LOAD
HDU2	HDU2–SDS2.5	THD62800	(2) 2x6	3,075lbs
HDU4	HDU4–SDS2.5	THD62800	(2) 2x6	4,420lbs
HDU5	HDU5–SDS2.5	THD62800	(2) 2x6	5,175lbs
HDU8	HDU8–SDS2.5	THD871000	(3) 2x6	7,870lbs
HDU11	HDU11–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	(4) 2x6 OR 6x6	9,535lbs
			(5) 2x6 OR 6x8	10,100lbs
HDU14	HDU14–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	6x6	14,925lbs
HDQ8	HDQ8–SDS3	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	(3) 2x6	9,230lbs
HHDQ11	HHDQ11–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	6x6	11,810lbs
HHDQ14	HHDQ14–SDS2.5	1"øx16" ASTM A307 ALL–THREAD ROD w/ SIMPSON SEX–XP EPOXY	6x6	13,710lbs

- ALL HOLDOWNS SHALL BE PRODUCED BY SIMPSON STRONG–TIE® ONLY. ANY PRODUCER OR PRODUCT SUBSTITUTIONS MUST BE APPROVED IN WRITING BY KIRK N. ELLIS & ASSOCIATES. PRIOR TO INSTALLATION.
- INSTALL ALL HOLDOWNS AND ANCHORS PER MANUFACTURER'S REQUIREMENTS AND SPECIFICATIONS
- SIMPSON SDS SCREW LENGTH ASSUMES DIRECT CONNECTION WITH POST SUPPORT. WHERE REQUIRED, INCREASE THE LENGTH OF SDS SCREWS THE THICKNESS OF SHEATHING MATERIAL AT INTERSECTING WALLS. (i.e. USE SDS25300 IN LIEU OF SDS25212 SCREWS AT ½" THICK SHEATHING FOR A WALL WITH CONT. SHEATHING INTERSECTING THE WALL REQUIRING THE HOLDOWN)
- HOLDOWNS SPECIFIED AT STEEL BEAMS SHALL USE SAME DIAMETER ANCHOR AS INDICATED ON THIS SCHEDULE, WELDED TO STEEL BEAM BELOW. SEE DETAIL 10/S4.2.
- USE ASTM A307 ALL–THREAD ROD WITH SIMPSON SET–XP EPOXY. PROVIDE MINIMUM 12" EMBEDMENT ON ALL ANCHORS AND INSTALL PER MANUFACTURER'S INSTRUCTIONS. TYP. U.N.O.

2

WALL TO WALL STRAP SCHEDULE

STRAP TYPE	FLOOR-TO-FLOOR CLEAR SPAN	REQ'D FASTENERS (TOTAL)	ALLOWABLE TENSION LOAD
MSTA49	≤18"	(26) 10d	2,020lbs
MSTC40	18"	(28) 16d SINKERS	2,290lbs
	16"	(36) 16d SINKERS	2,945lbs
	0"	(52) 16d SINKERS	4,265lbs
MSTC52	18"	(44) 16d SINKERS	3,595lbs
	16"	(48) 16d SINKERS	3,925lbs
	0"	(62) 16d SINKERS	4,745lbs
MSTC66	18"	(64) 16d SINKERS	5,460lbs
	16"	(68) 16d SINKERS	5,800lbs
	0"	(74) 16d SINKERS	5,800lbs
CMST14	≤18"	(66) 16d	6,490lbs
CMST12	≤18"	(84) 16d	9,215lbs

- ALL STRAP TIE HOLDOWNS SHALL BE PRODUCED AND MANUFACTURED BY SIMPSON STRONG–TIE ONLY. ANY PRODUCT OR MANUFACTURER SUBSTITUTIONS MUST BE APPROVED IN WRITING BY KIRK N. ELLIS & ASSOCIATES PRIOR TO INSTALLATION.
- INSTALL ALL STRAPS PER MANUFACTURER'S INSTRUCTIONS.
- NAILS ARE NOT REQUIRED IN FLOOR–TO–FLOOR CLEAR SPAN AND ARE NOT COUNTED IN THE REQUIRED TOTAL FASTENERS
- ALL STRAPS REQUIRE (2) 2x STUDS AT CENTERLINE OF STRAP.

SHEATHING & SHEAR NAILING SCHEDULE FOR SHEAR WALLS

SYMBOL	PLYWOOD AND NAILING REQUIREMENT
6	APPLY ½" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **6" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE 2" NOMINAL OR THICKER AT ABUTTING PANEL EDGES.
4	APPLY ½" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **4" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE 2" NOMINAL OR THICKER AT ABUTTING PANEL EDGES.
3	APPLY ½" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **3" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
2	APPLY ½" OSB WOOD SHEATHING AND NAIL WITH 10d COMMON x 2¼" NAILS AT **2" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
44	APPLY ½" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2¼" NAILS AT **4" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
33	APPLY ½" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2¼" NAILS AT **3" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.
22	APPLY ½" OSB WOOD SHEATHING **BOTH SIDES OF WALL** AND NAIL WITH 10d COMMON x 2¼" NAILS AT **2" O.C.** AT PANEL EDGES AND 10" O.C. FIELD NAILING. BLOCK SOLIDLY ALL EDGES OF PLYWOOD. FRAMING SHALL BE **3" NOMINAL** OR THICKER AT ABUTTING PANEL EDGES.

WHERE ALL SHEARWALLS INTERSECT AT A CORNER, PROVIDE THE TIGHTER OF THE TWO NAILING SCHEDULES TO BOTH SIDES OF THE CORNER, TYP.

POWDER DRIVEN PIN ANCHORAGE SPACING

SYMBOL	REQUIREMENT
9	(2) ROWS HILTI X–U72 PINS AT 18" O.C. MAX. STAGGERED TO NET 9" O.C. MAX. IN BOTTOM LAYER OF DOUBLE 2x6 BOTTOM PL.
6	(2) ROWS HILTI X–U72 PINS AT 12" O.C. MAX. STAGGERED TO NET 6" O.C. MAX. IN BOTTOM LAYER OF DOUBLE 2x6 BOTTOM PL.
5	(2) ROWS HILTI X–U72 PINS AT 10" O.C. MAX. STAGGERED TO NET 5" O.C. MAX. IN BOTTOM LAYER OF DOUBLE 2x6 BOTTOM PL.

NOTE:
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ISSUED FOR BUILDING DEPARTMENT PLAN CHECK
May 27, 2009

NOTE:
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KIRK N. ELLIS & ASSOCIATES
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PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

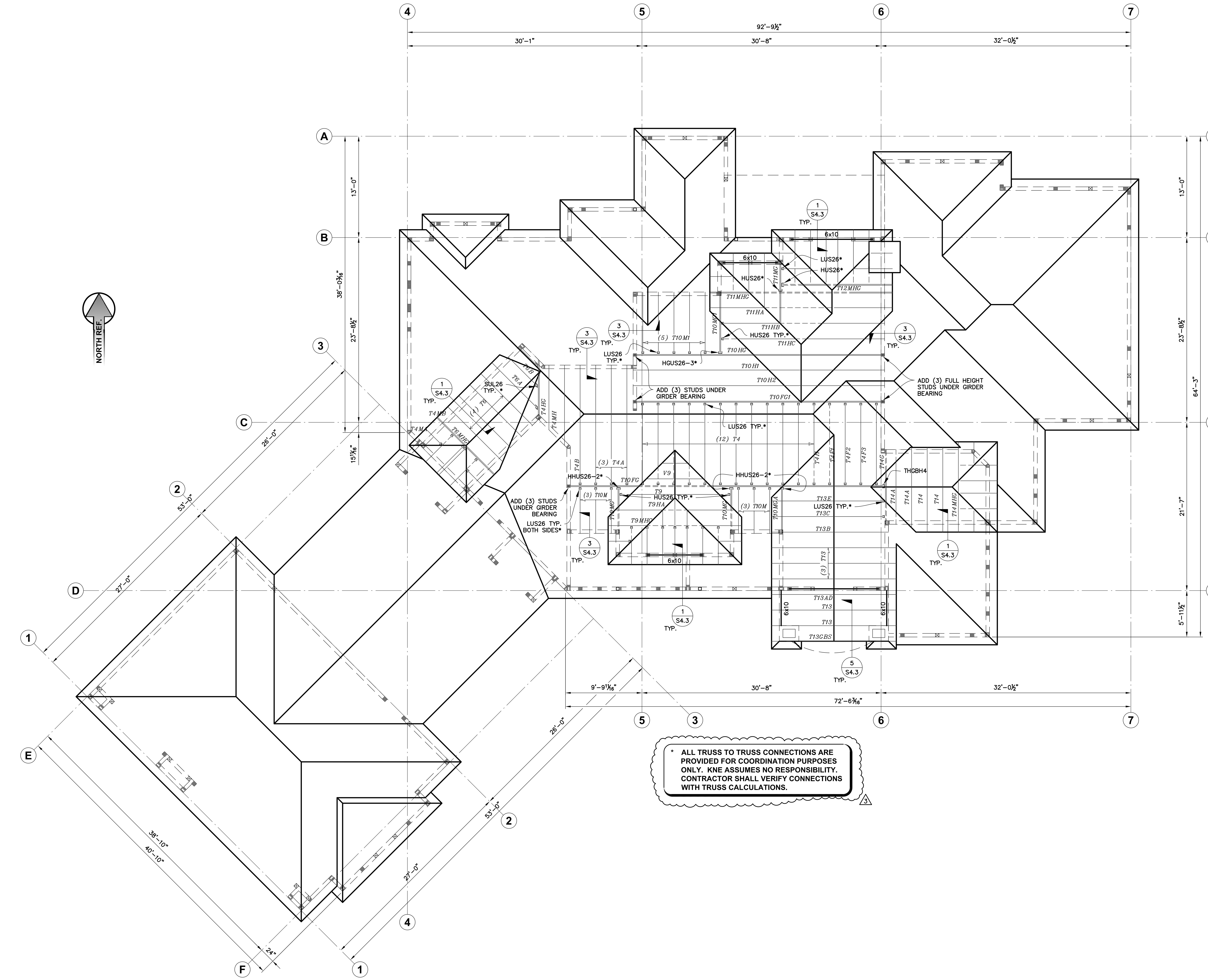
Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:

DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
DESIGNED: TRH/KNE
CHECKED: KNE
KNE JOB NO.: 1309

SHEET TITLE: STRUCTURAL SECOND FLOOR WALL FRAMING PLAN

SHEET NO. S2.4
OF 15 SHEETS



ROOF FRAMING PLAN
SCALE: 3/16" = 1'-0"

1
S2.5

4007SPLANS

ROOF FRAMING NOTES

1. ALL GRIDLINES SHOWN ARE TO THE FACE OF STUD TYP., U.N.O.
2. EXTERIOR WALLS ABOVE AND INTERIOR STRUCTURAL BEARING WALLS SHALL CONSIST OF 2x6 D.F.#2 WOOD STUDS AT 16" O.C. (MAX.), TYP., U.N.O.
3. USE ⅝" APA RATED (40/20) CDX EXP. 1 STRUCTURAL SHEATHING WITH 10d COMMON x2⅜" NAILS AT 6" O.C. EDGE AND 12" O.C. FIELD, TYP. U.N.O. IN ADDITION, PROVIDE NAILING AS INDICATED IN DETAIL 6/S1.2, TIGHTER NAILING REQUIREMENTS SHALL ALWAYS CONTRAOL WHERE THERE IS A CONFLICT.
4. CONTRACTOR SHALL PROVIDE POSITIVE ATTIC VENTILATION PER THE 2006 I.B.C. AND APPLICABLE WASHOE COUNTY CODES.
5. COORDINATE DIMENSIONS, OPENING AND INFORMATION WITH THE ARCHITECTURAL DRAWINGS AS REQUIRED. REPORT AND DISCREPANCIES TO STRUCTURAL ENGINEER.

PLYWOOD EDGE NAILING AND BLOCKING INFORMATION

BLOCK ALL PLYWOOD EDGES WITH 3x FLAT BLOCKING AND NAIL WITH 10d COMMON NAILS AT 6" O.C. BOUNDARY AND EDGE, AND 10" O.C. FIELD.

BLOCK ALL PLYWOOD EDGES WITH 3x FLAT BLOCKING AND NAIL WITH 10d COMMON NAILS AT 4" O.C. BOUNDARY AND EDGE, AND 10" O.C. FIELD.

BLOCK ALL PLYWOOD EDGES WITH 3x FLAT BLOCKING AND NAIL WITH 10d COMMON NAILS AT 3½" O.C. BOUNDARY AND EDGE, AND 10" O.C. FIELD.

TRUSS DESIGN INFORMATION

	DEAD LOAD MAXIMUM	DEAD LOAD MINIMUM	SNOW LOAD	WIND UPLIFT (2)
TOP CHORD	22.0 psf	10.0 psf	20 psf	29.6 psf
BTM. CHORD	10.0 psf	7.0 psf	**10 psf	N/A

** DESIGN BOTTOM CHORD FOR 10 PSF NON-CONCURRENT LIVE LOAD AND 200LB MOVING POINT LOAD

1. TRUSS WEB CONFIGURATIONS SHOWN ARE REPRESENTATIVE ONLY. ACTUAL TRUSS CONFIGURATIONS TO BE DETERMINED BY TRUSS COMPANY. ALL TOP CHORDS SHALL BE 2x6 TYP. U.N.O.
2. NORMAL FORCE TO ROOF ELEMENTS AND COMPONENTS PER 2006 IBC SEC. 1620
3. ALL TRUSS TO TRUSS CONNECTIONS ARE PER THE TRUSS MANUFACTURER AND MUST BE GOOD FOR BOTH GRAVITY AND WIND UPLIFT LOADS.
4. ALL LOADS SHOWN ABOVE ARE PROJECTED. EXCEPT THE WIND LOAD.
5. PER WASHOE COUNTY LOCAL REQUIREMENTS, INCREASE THE SNOW LOADS AT THE EAVES BY A FACTOR OF 2.0

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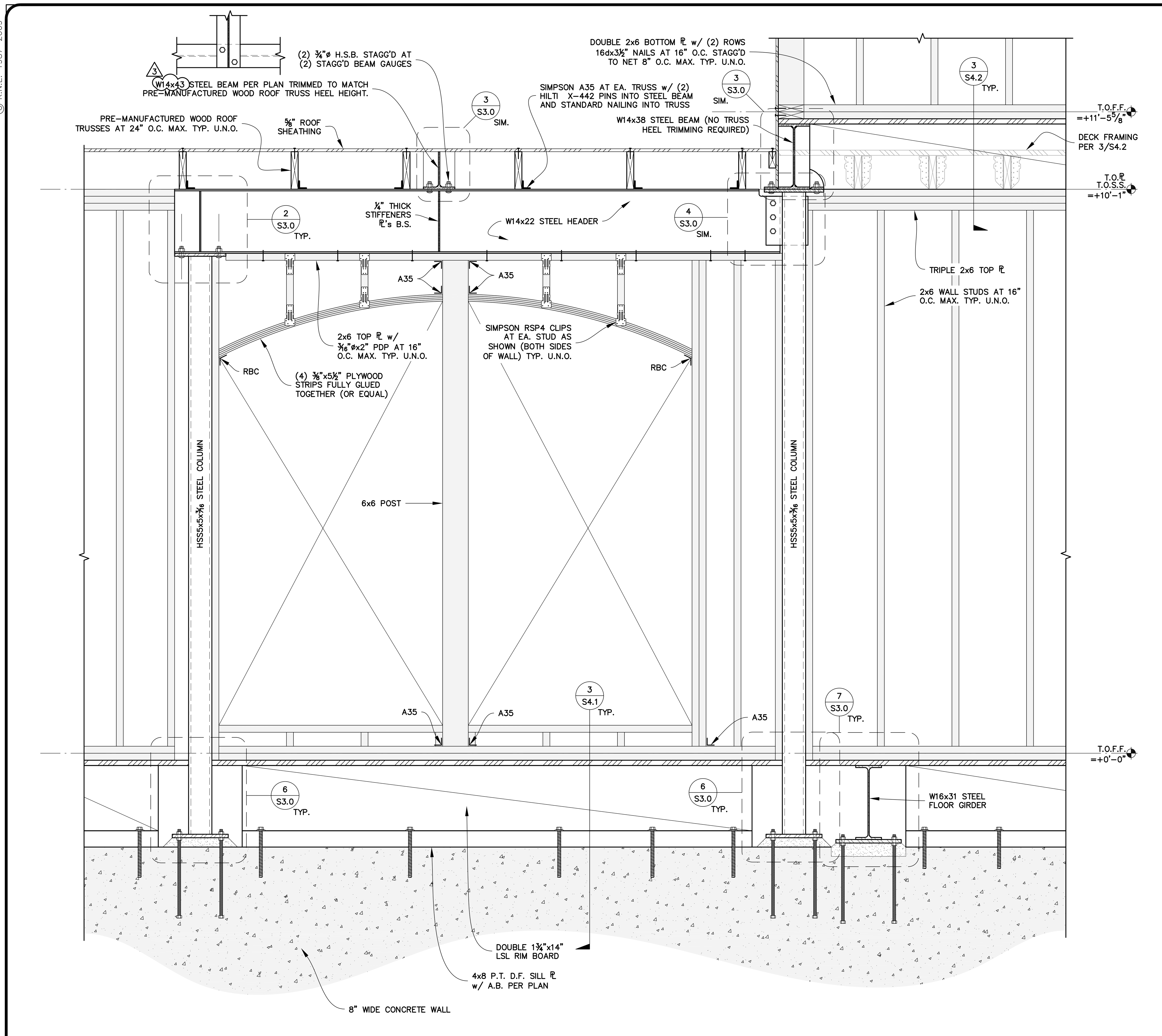
REVISIONS:
1. 08/31/2009 ARCH'L COORD.
2. 08/31/2009 BUILDING DEPT.
3. 09/10/2009 BUILDING DEPT.

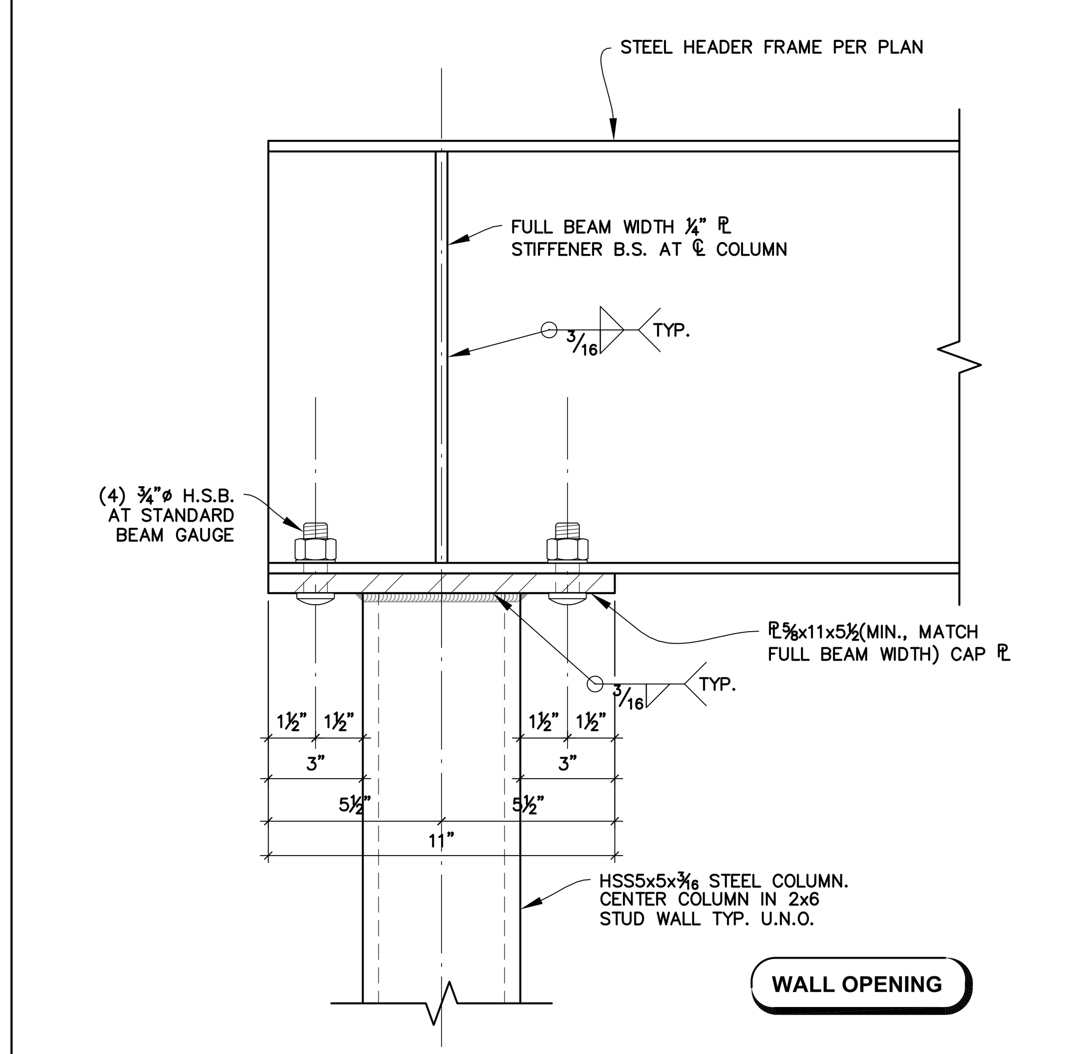
DATE:	05/27/2009
SCALE:	AS SHOWN
DRAWN:	BLW
DESIGNED:	TRH/KNE
CHECKED:	KNE
KNE JOB NO.:	1309

SHEET TITLE: STRUCTURAL ROOF FRAMING PLAN

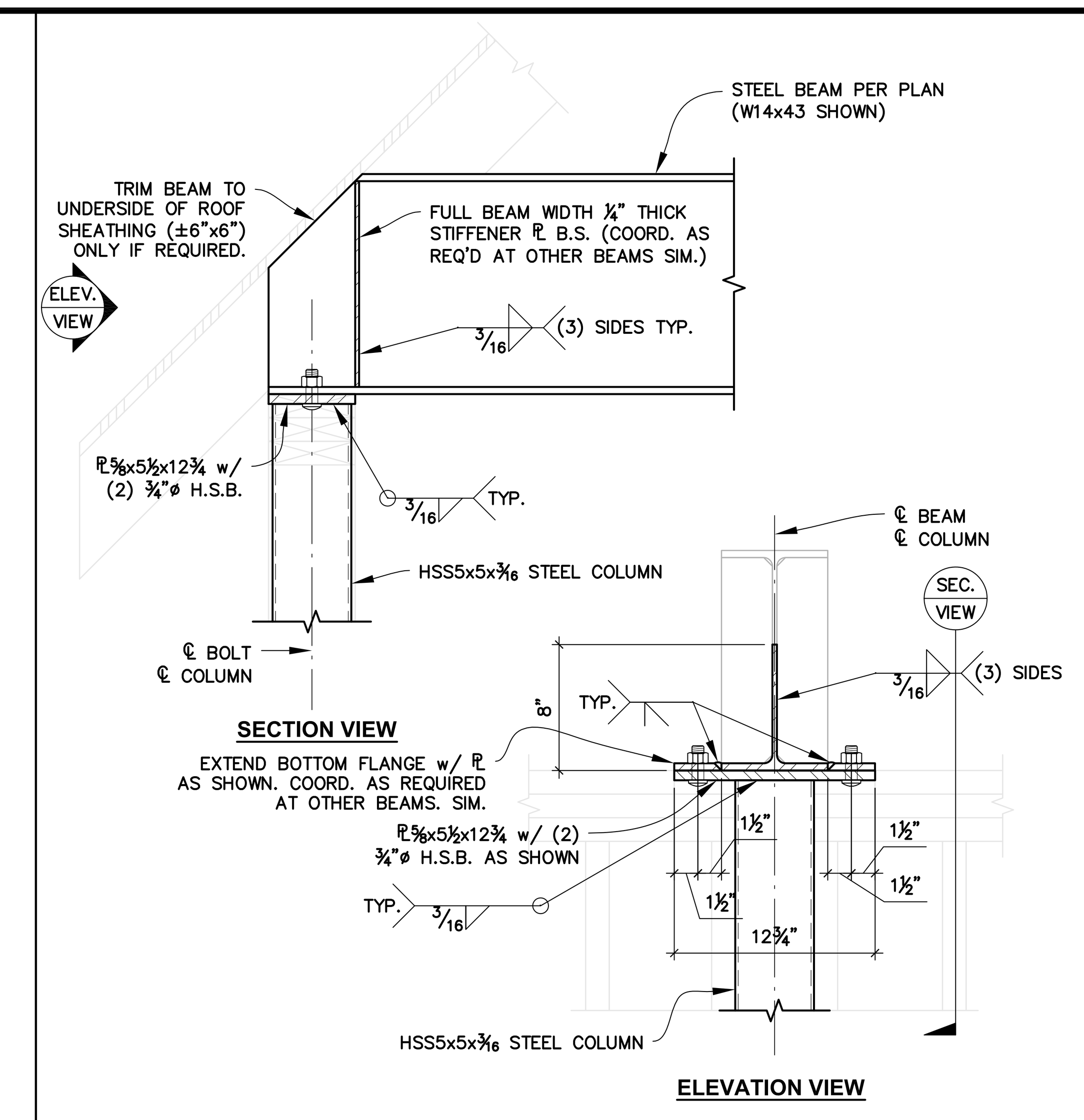
SHEET NO. S2.5 OF 15 SHEETS



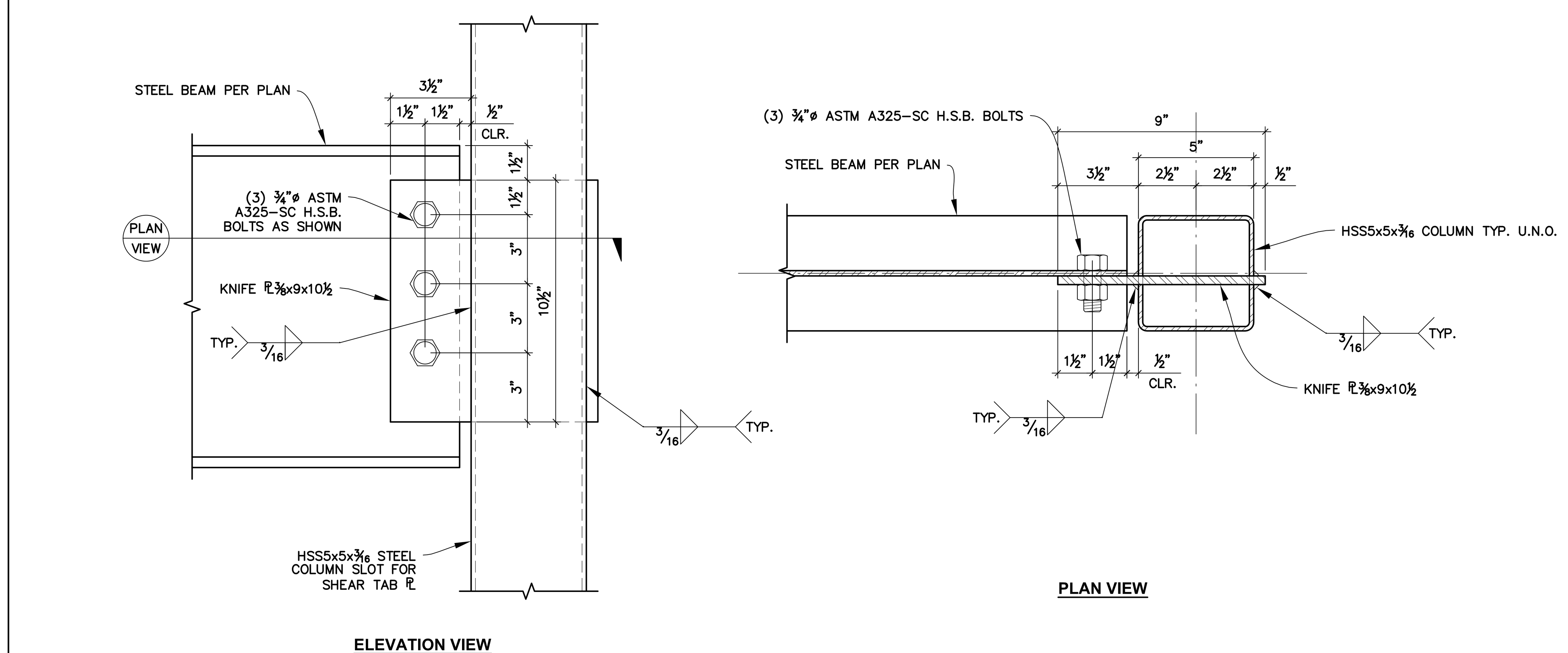
PARTIAL STRUCTURAL FRAMING ELEVATION 1/S3.0
SCALE: 1" = 1'-0" 1309SELEVATION1



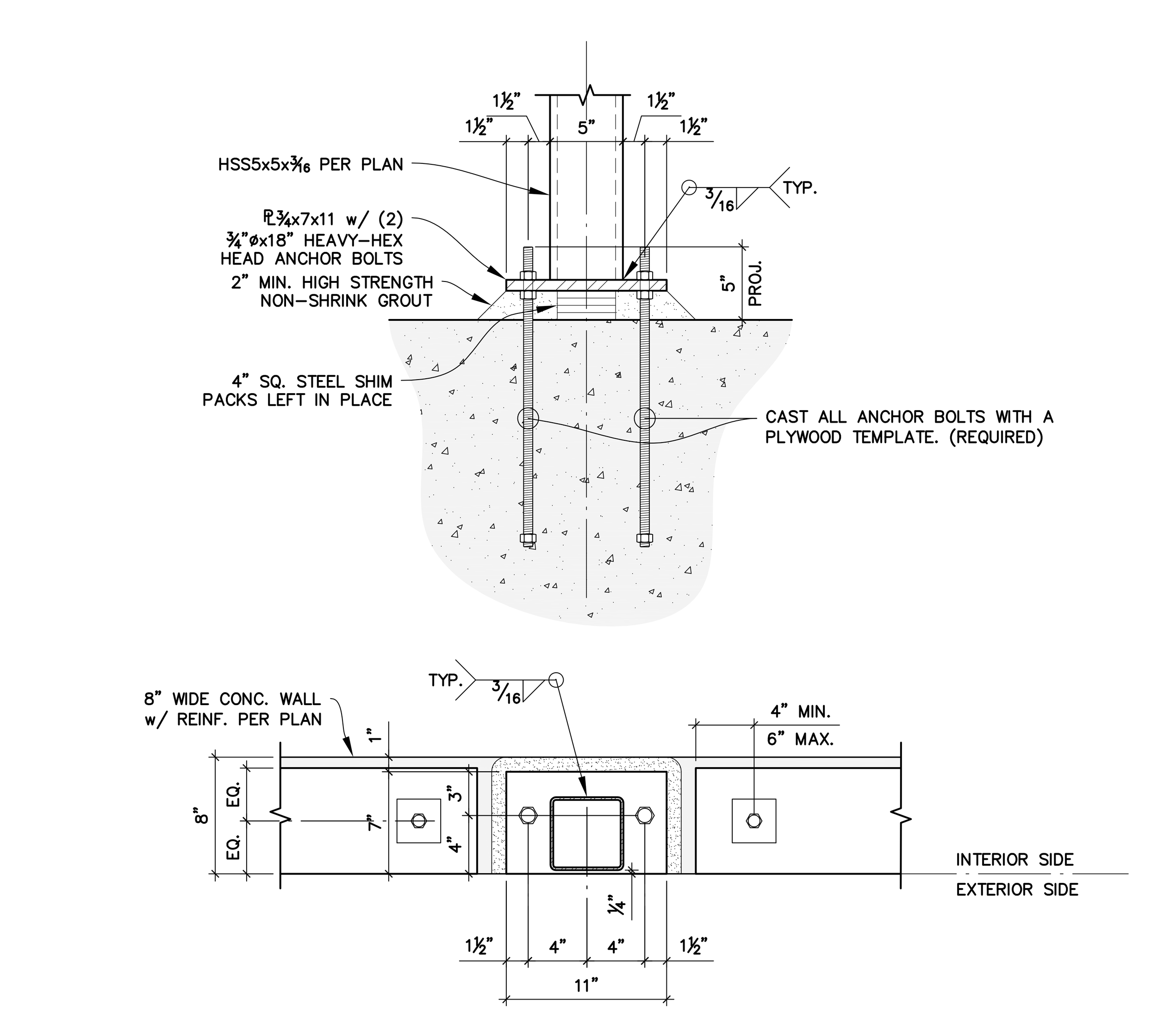
TYPICAL STEEL IN-WALL HEADER BEAM TO COLUMN CONNECTION DETAIL 2/S3.0
SCALE: 3" = 1'-0" 1309DT30



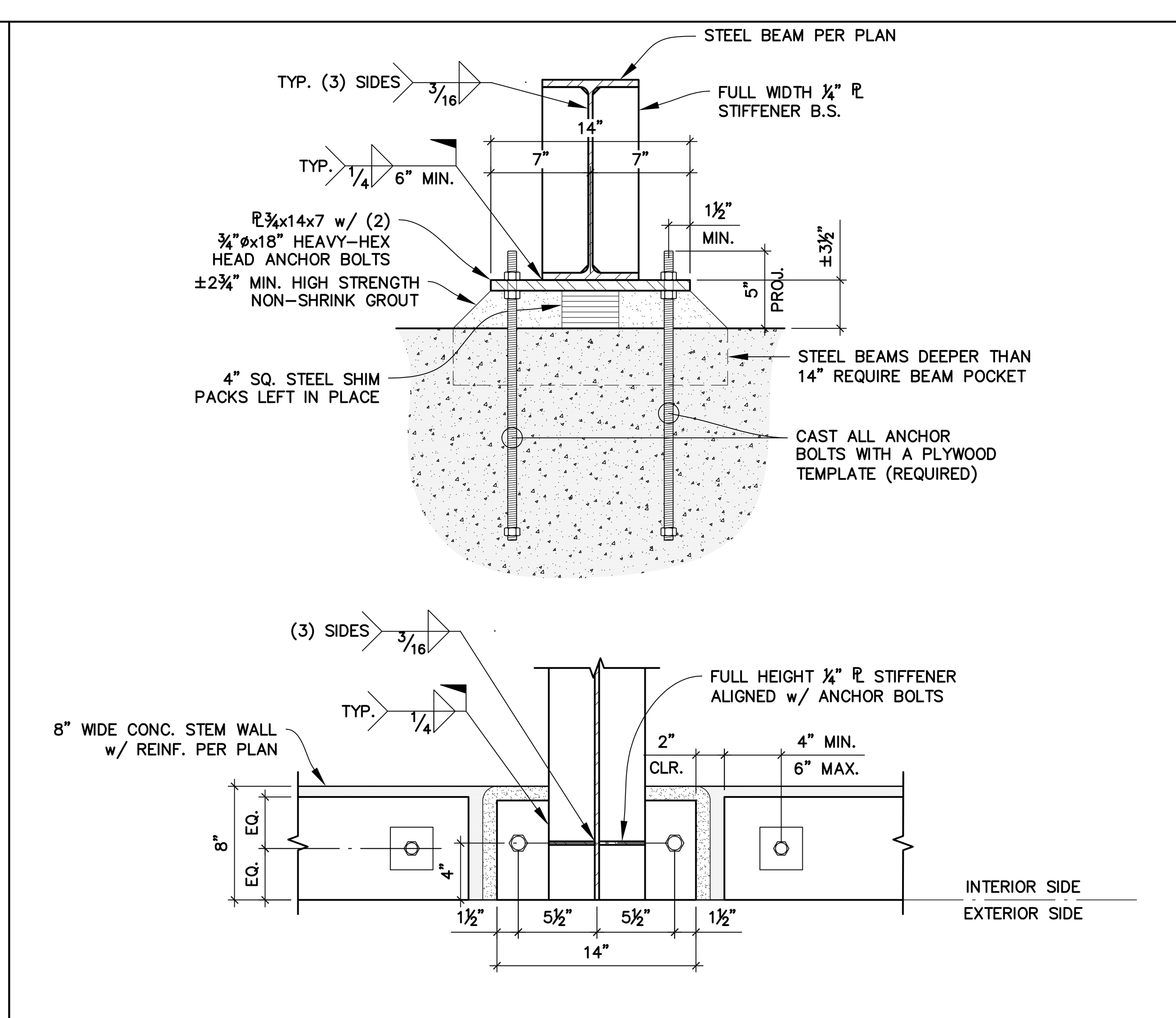
TYP. STEEL BEAM TO PERPENDICULAR WALL AT TRUSS CONNECTION DETAIL 3/S3.0
SCALE: 1½" = 1'-0" 1309DT33



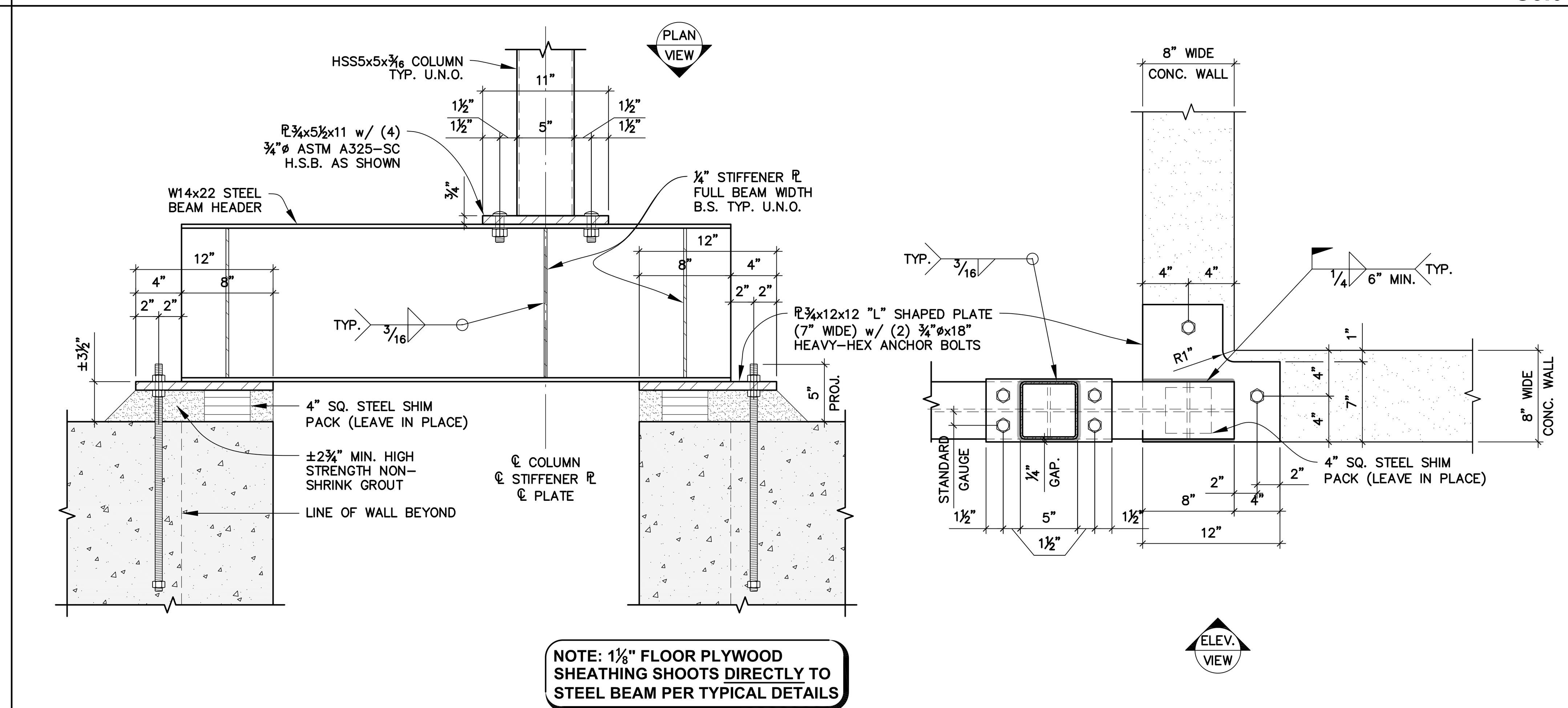
TYP. STEEL BEAM TO HSS COLUMN CONNECTION DETAIL 4/S3.0
SCALE: 3" = 1'-0" 1309DT35



TYP. STEEL COLUMN TO CONC. WALL CONNECTION DETAIL 6/S3.0
SCALE: 1½" = 1'-0" 1309DT37



TYP. WIDE FLANGE BEAM TO CONC. WALL CONNECTION 7/S3.0
SCALE: 1½" = 1'-0" 1309DT38



TYP. STEEL BEAM HEADER DETAIL 8/S3.0
SCALE: 1½" = 1'-0" 1309DT46

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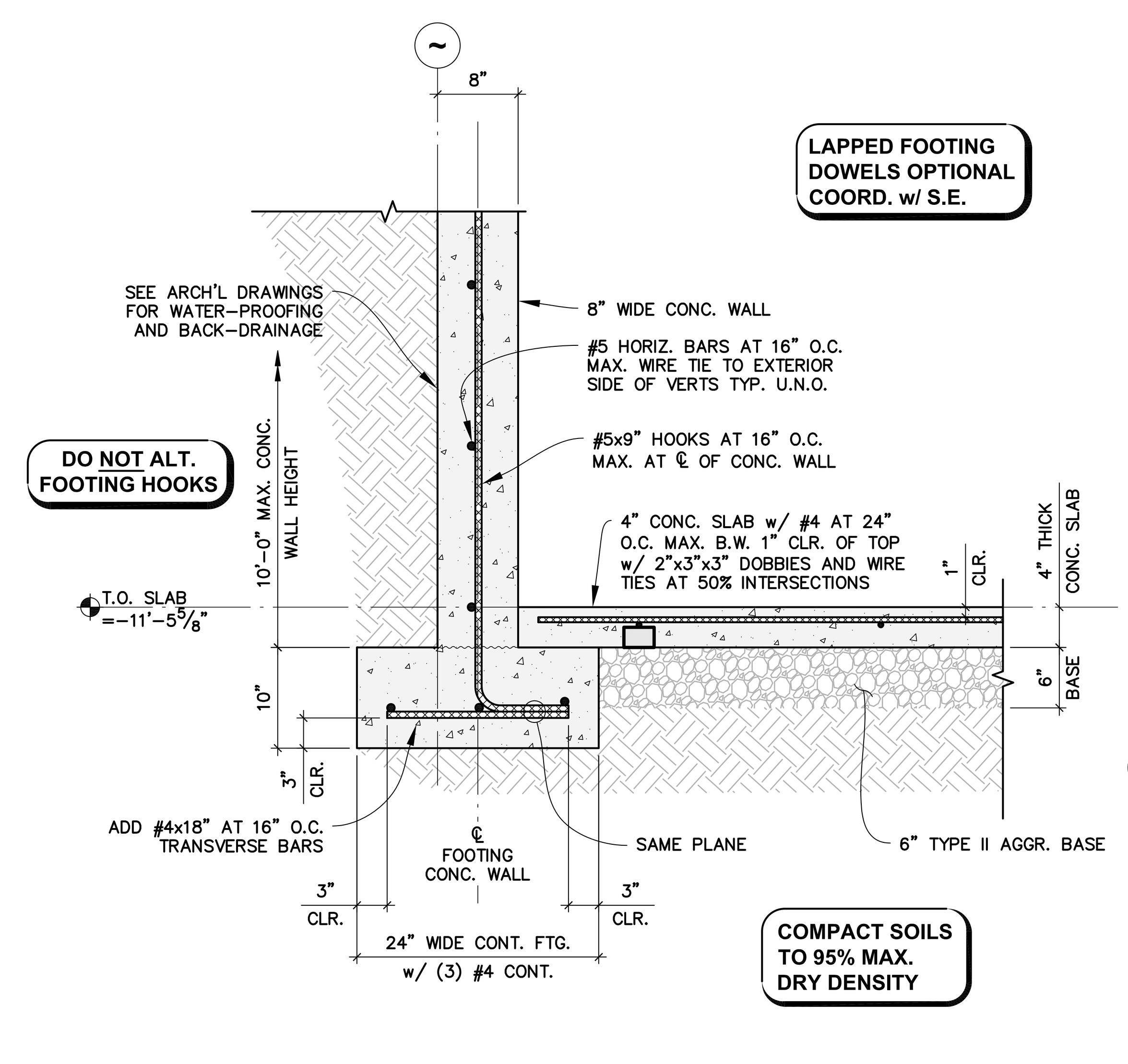
REVISIONS: 1 09/10/2009 BUILDING DEPT.

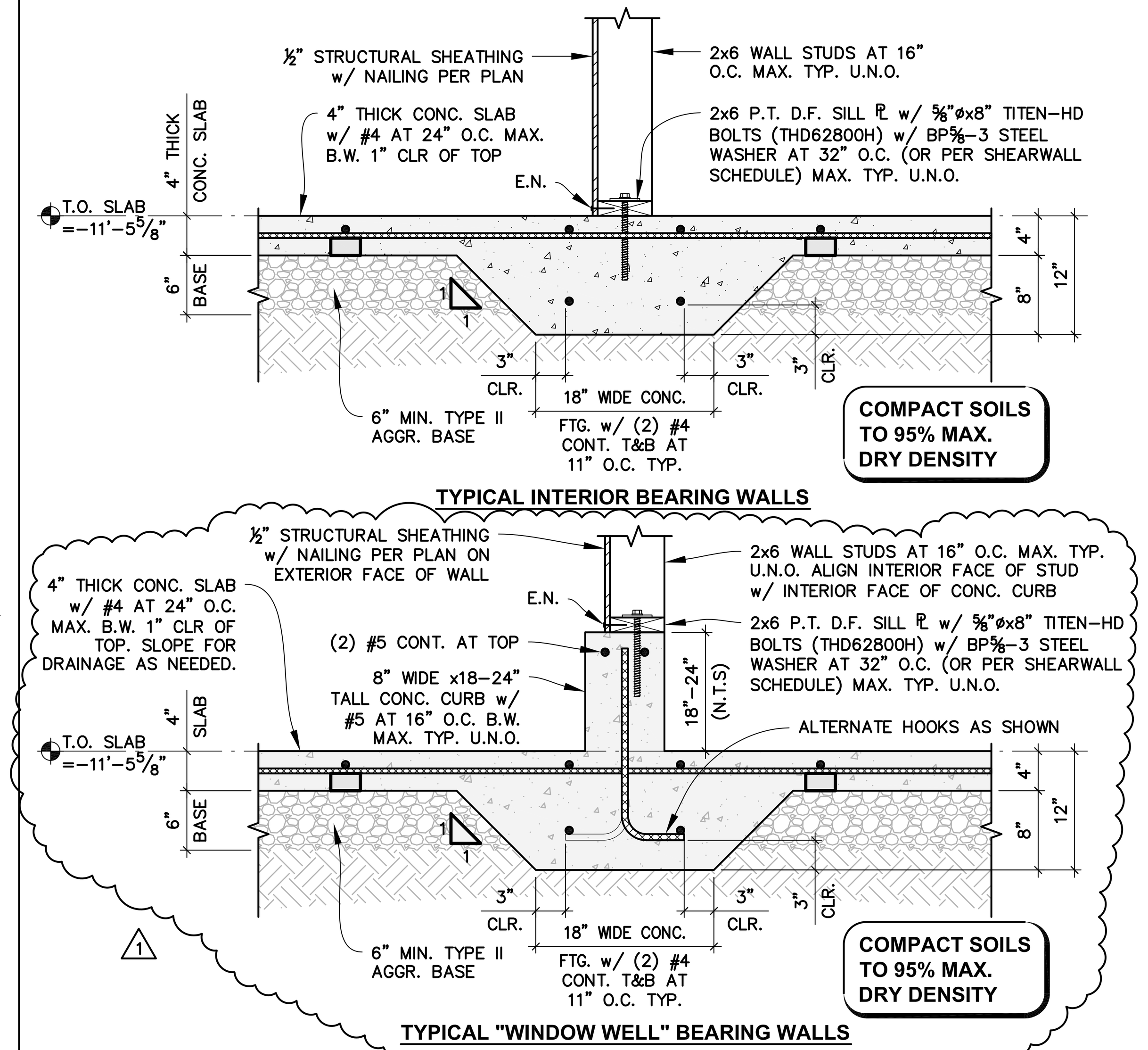
DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
DESIGNED: TRH/KNE
CHECKED: KNE
JOB NO.: 1309

SHEET TITLE: PARTIAL STRUCTURAL ELEVATION AND STEEL CONNECTION DETAILS

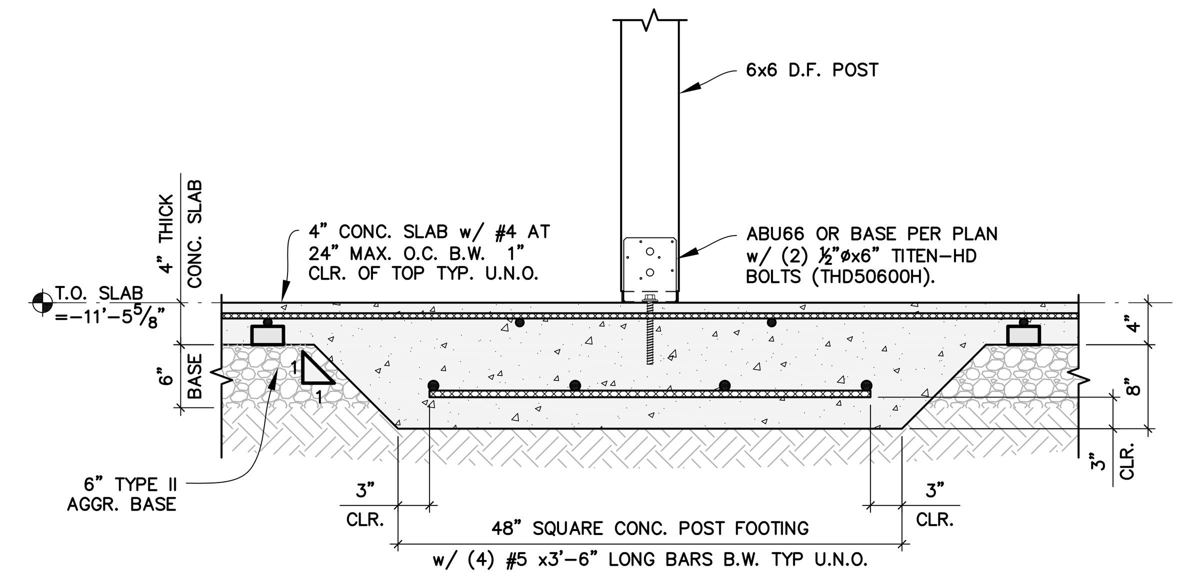
SHEET NO. S3.0
OF 15 SHEETS



TYPICAL CONC. WALL FOOTING DETAIL 1 / S4.0
SCALE: 1" =1'-0" 1309DT01

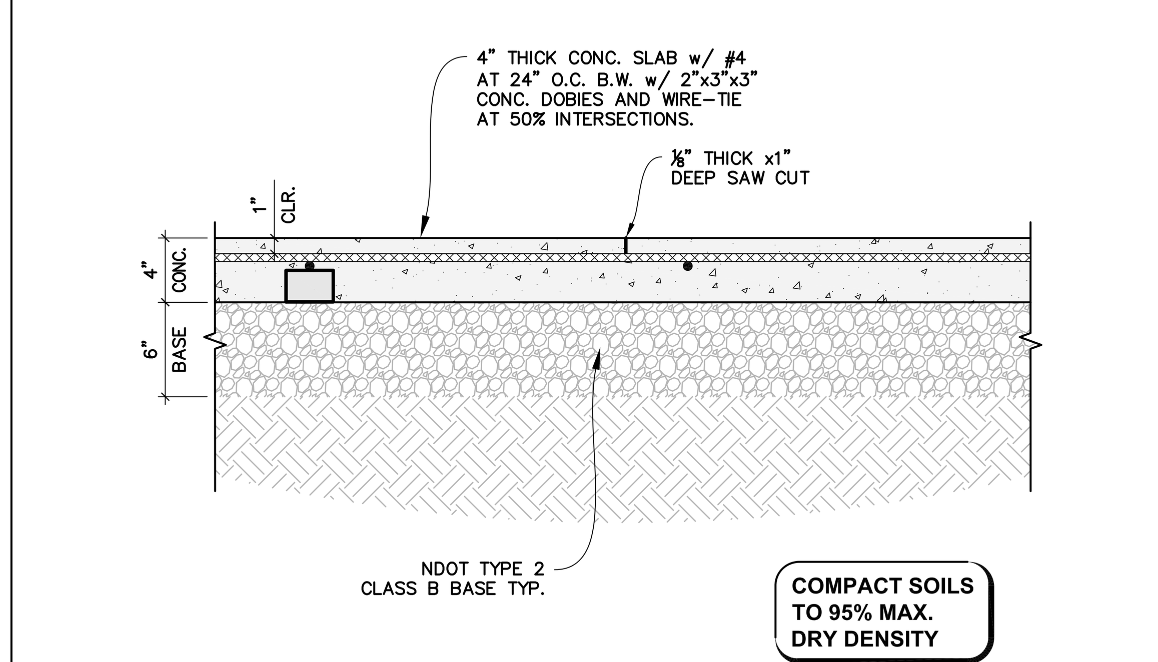


TYPICAL WOOD BEARING WALL CONNECTION DETAIL 2 / S4.0
SCALE: 1" = 1'-0" 1309DT02



TYPICAL 6x6 WOOD POST FOOTING DETAIL 3 / S4.0
SCALE: 1" = 1'-0" 1309DT03

MAXIMUM ±9'-0" x ±9'-0" SQUARES FOR SAWCUT SECTIONS AS SHOWN PER PLAN



TYP. SAWCUT JOINT 4 / S4.0
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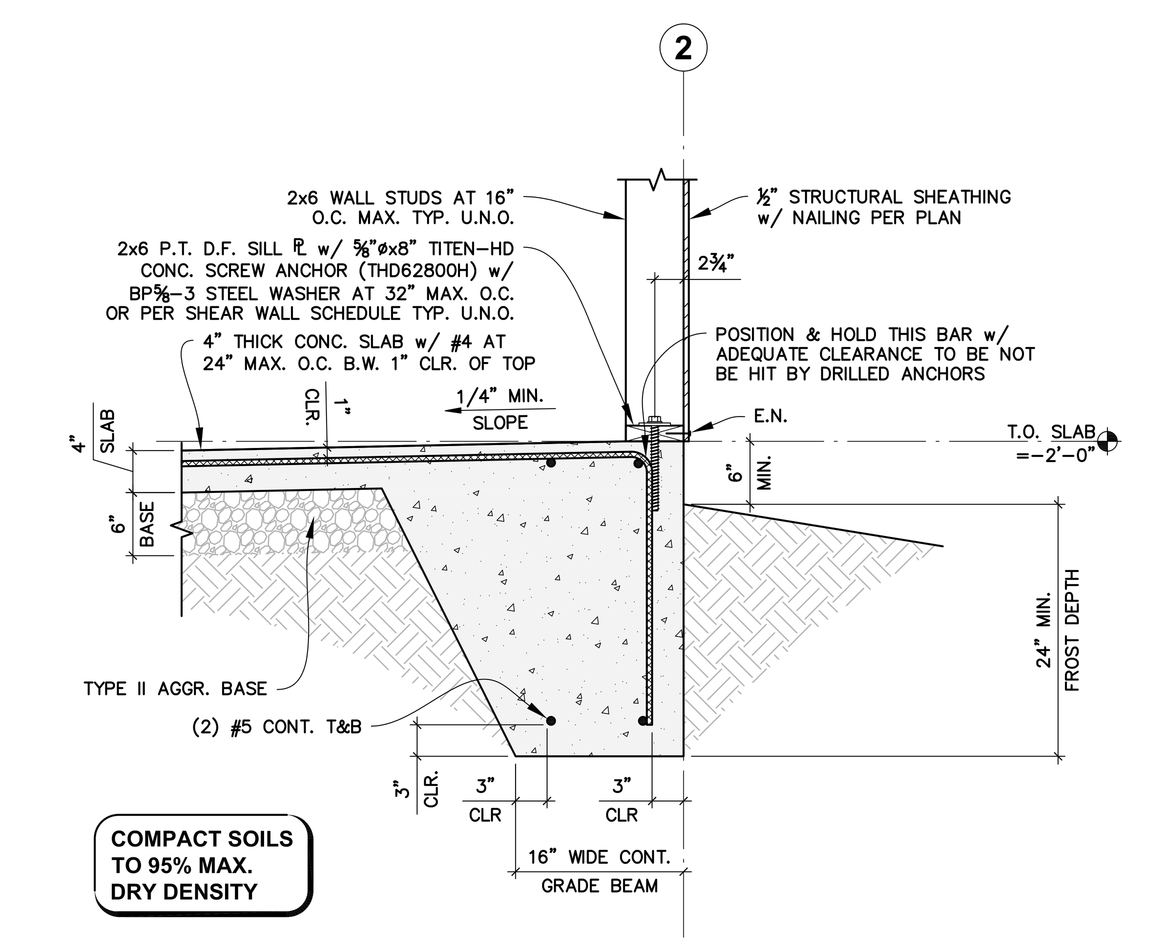
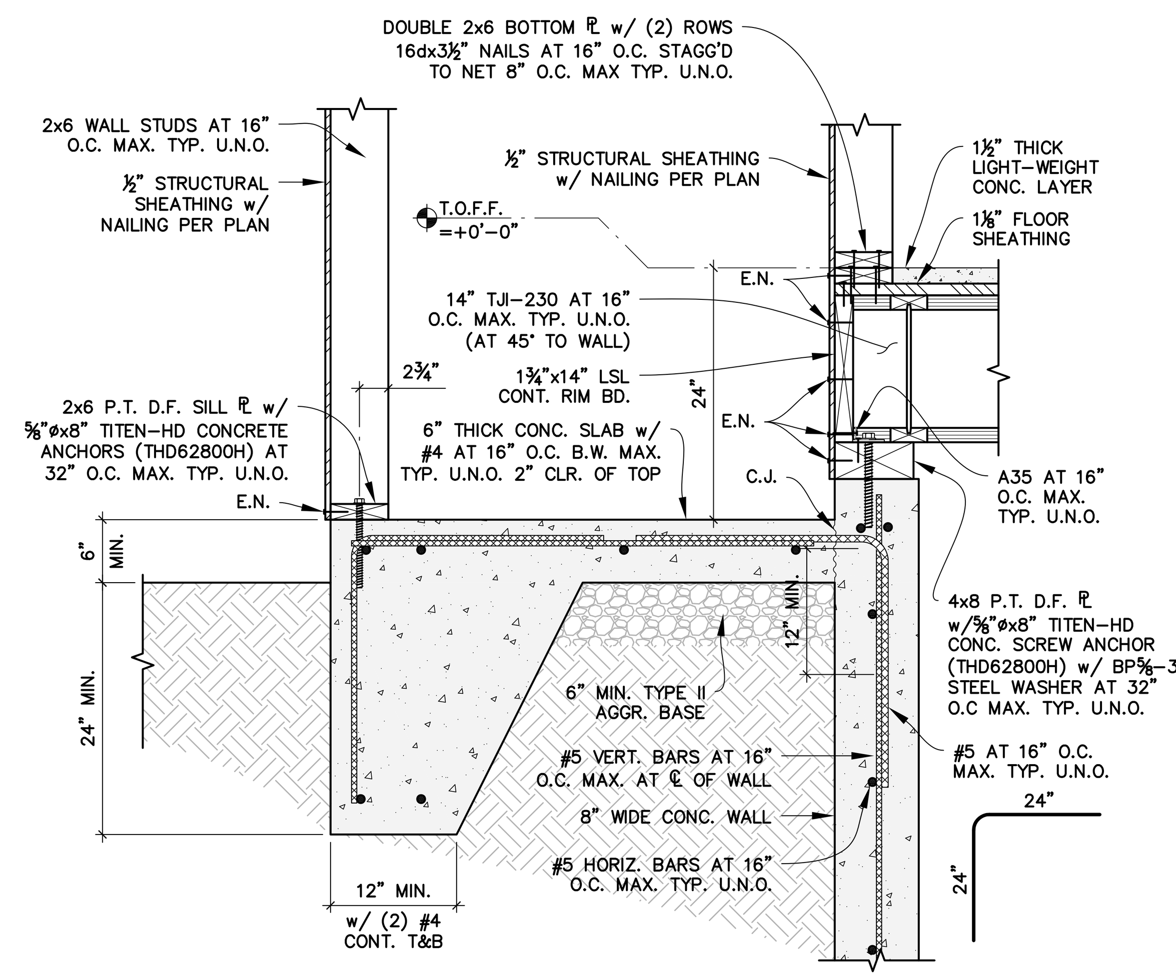
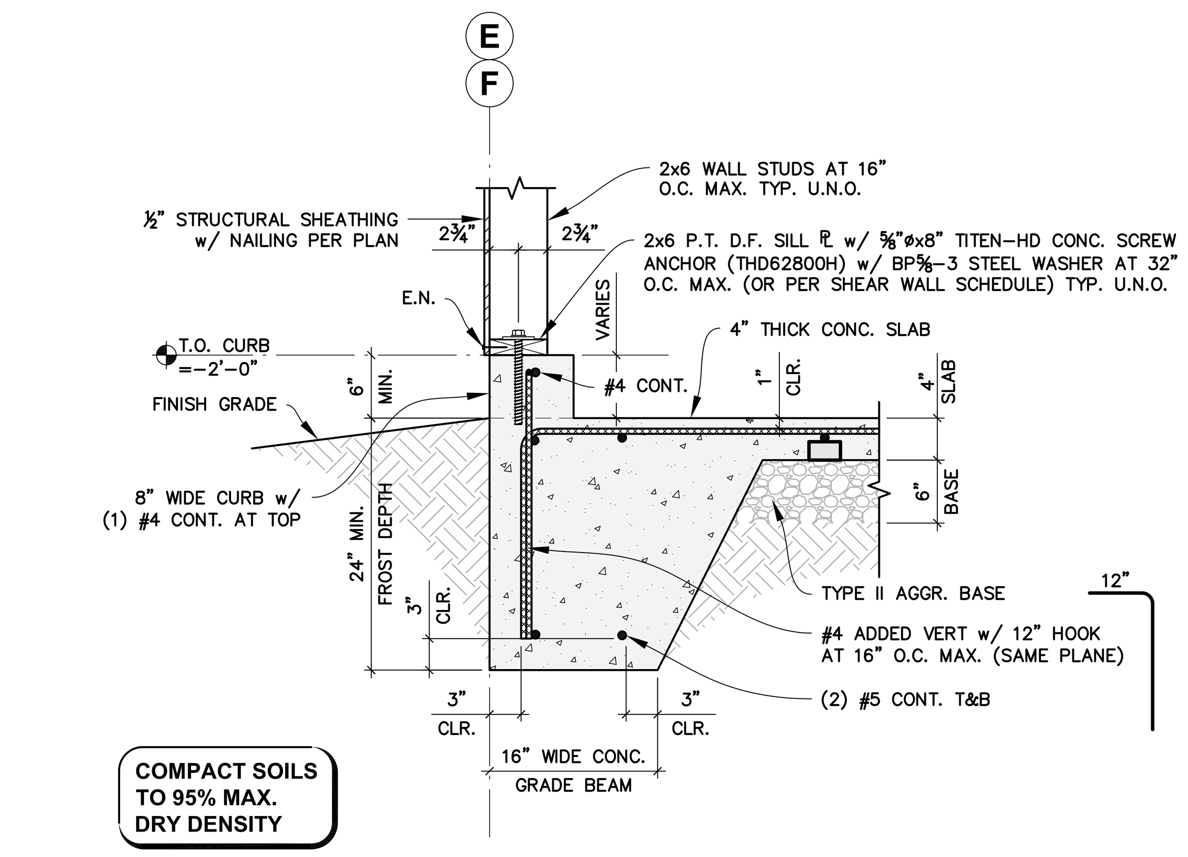


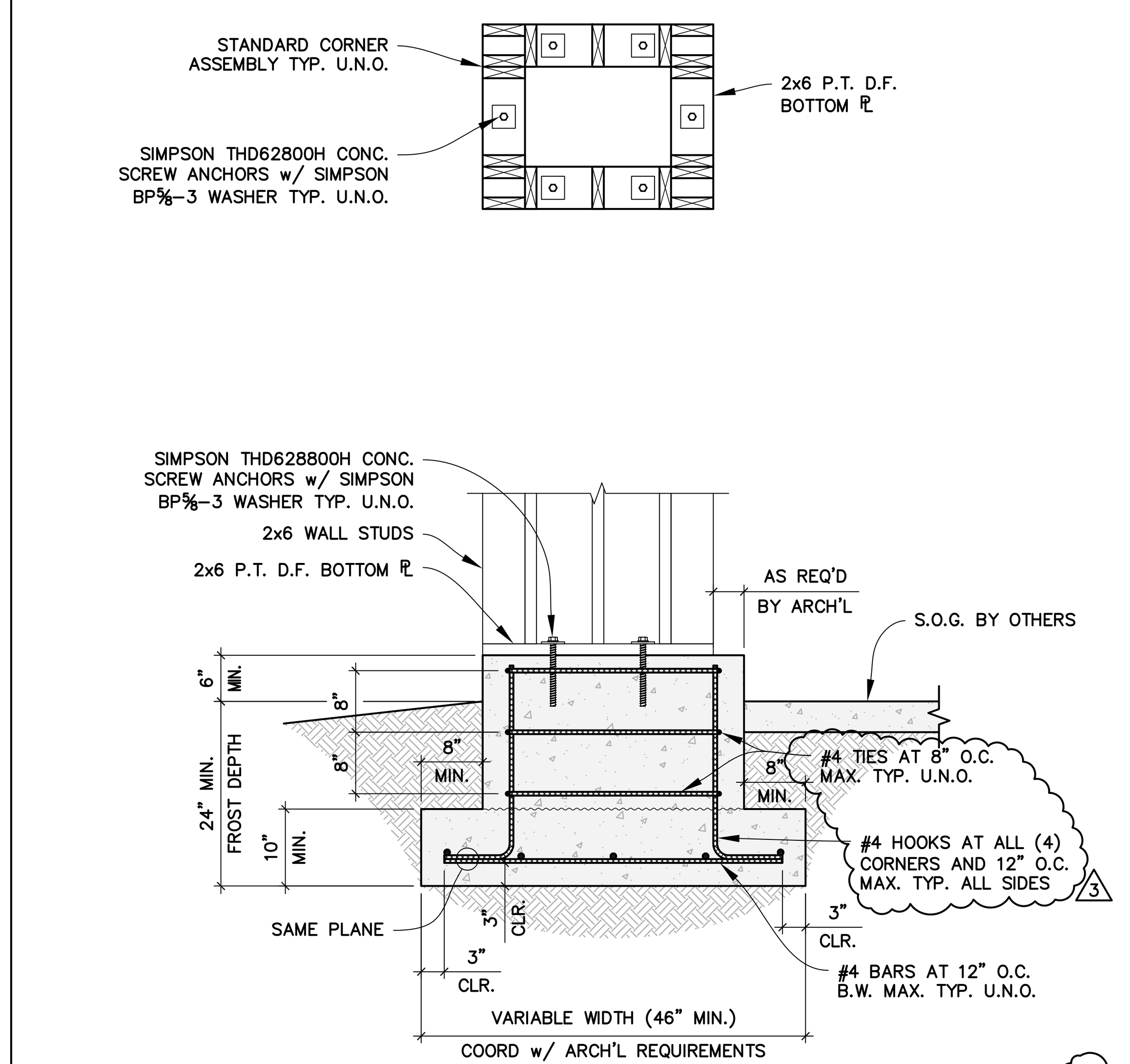
TYPICAL GARAGE FOOTING AT CARPORT SLAB 5 / S4.0
SCALE: 1" = 1'-0" 1309DT09



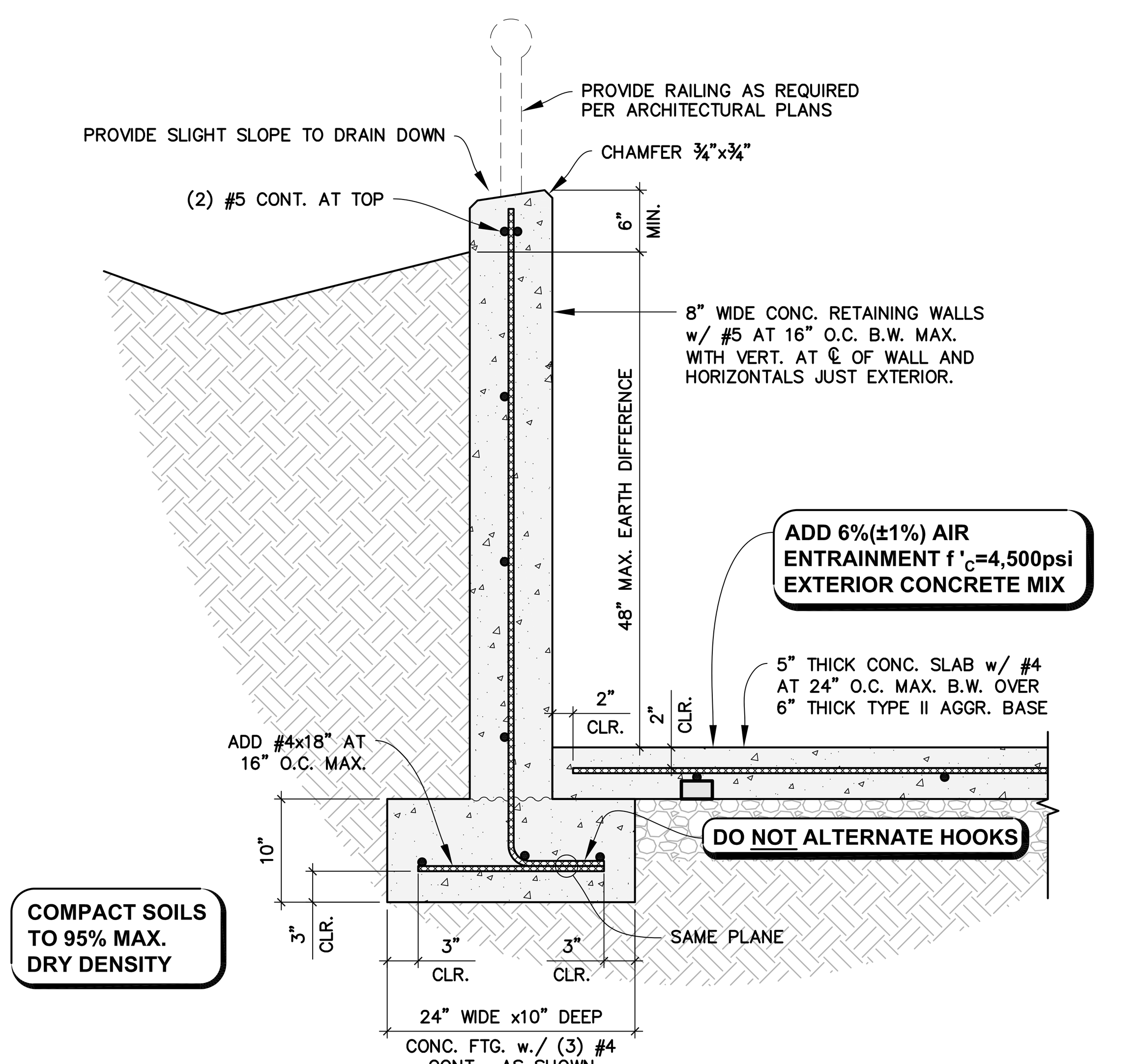
TYPICAL SLAB-ON-GRADE SECTION DETAIL 6 / S4.0
SCALE: 1" = 1'-0" 1309DT04



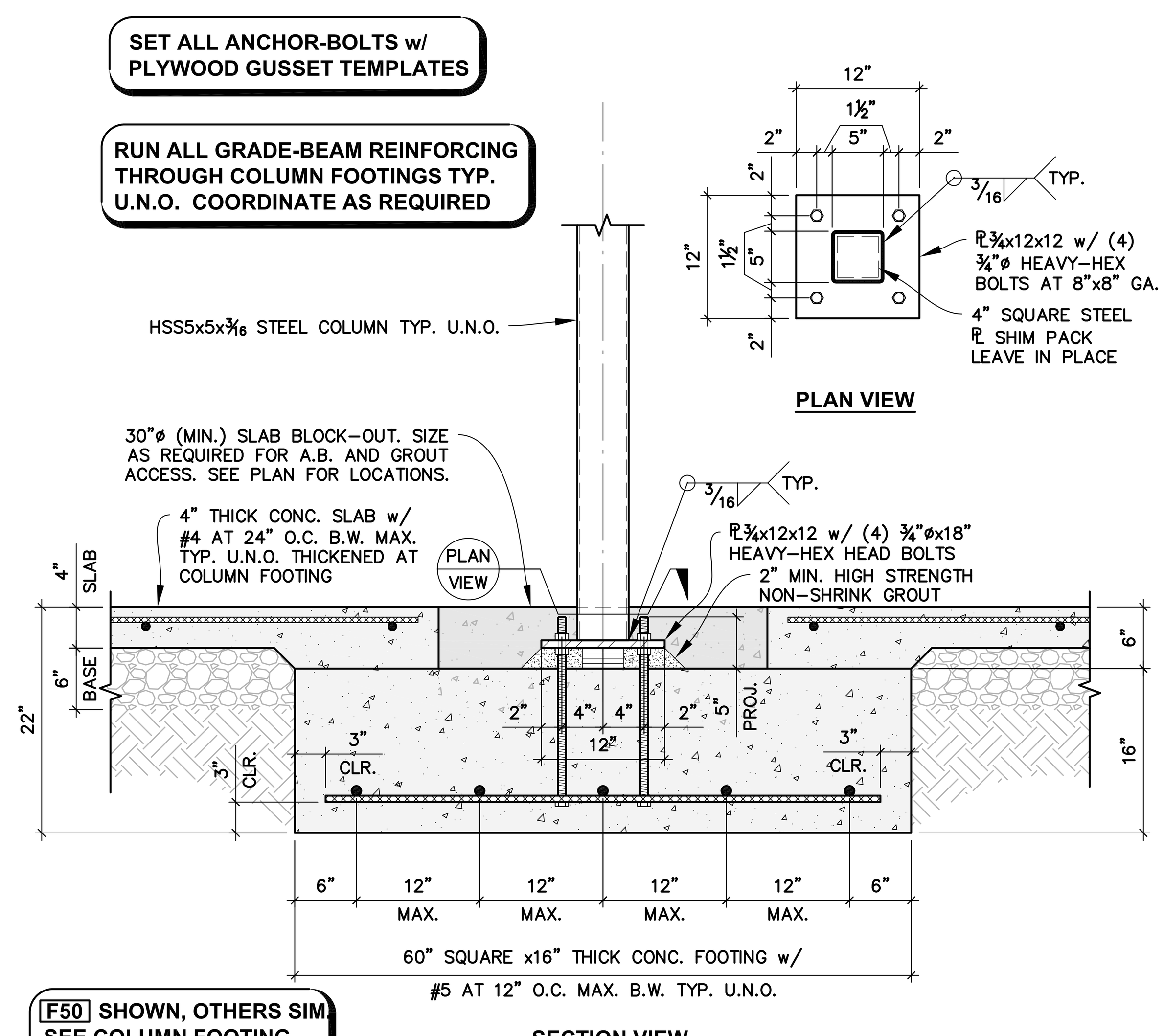
TYPICAL MAIN LEVEL GARAGE GRADE BEAM 7 / S4.0
SCALE: 1" = 1'-0" 1309DT10



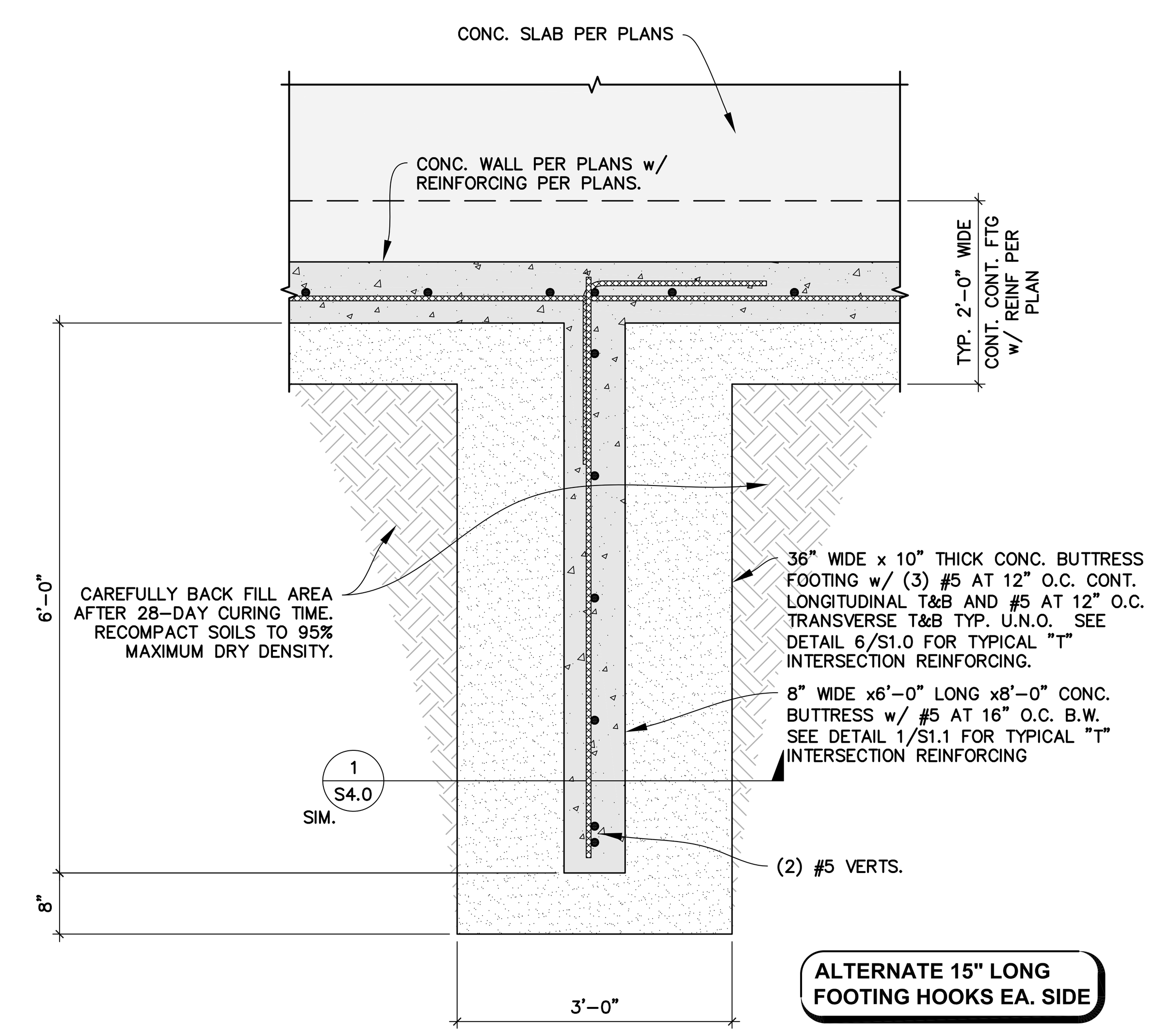
TYP. PEDESTAL DETAIL 8 / S4.0
SCALE: ¾" = 1'-0" 1309DT54



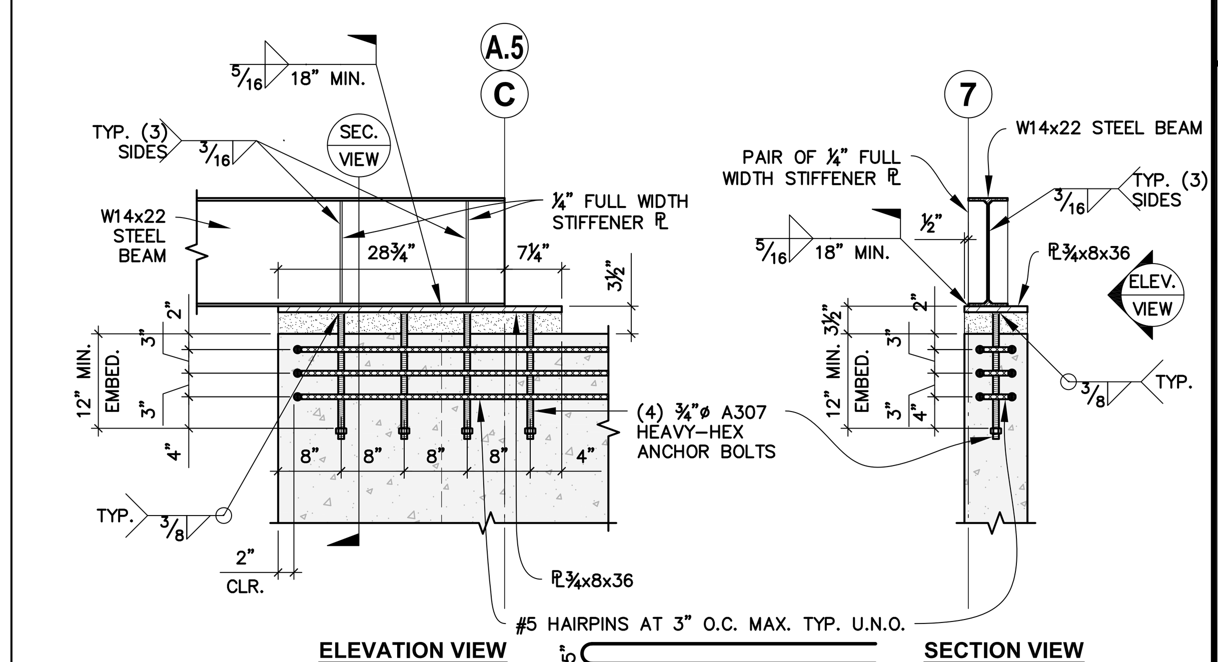
TYP. 4'-0" MAX. RETAINING WALL 9 / S4.0
SCALE: 1" = 1'-0" 1309DT12



TYPICAL STEEL COLUMN FOOTING DETAIL 10 / S4.0
SCALE: 1" = 1'-0" 1309DT39



TYPICAL BUTTRESS CONSTRUCTION DETAIL PLAN VIEW 11 / S4.0
SCALE: ¾" = 1'-0" 1309DT44



TYP. STEEL BEAM DRAG CONNECTION 12 / S4.0
SCALE: ¾" = 1'-0" 1309DT63

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DATE: 05/27/2009
SCALE: AS SHOWN
DRAWN: BLW
CHECKED: TRH/KNE
KNE JOB NO. 1309

SHEET TITLE: STRUCTURAL BASEMENT DETAILS

SHEET NO. S4.0 OF 15 SHEETS

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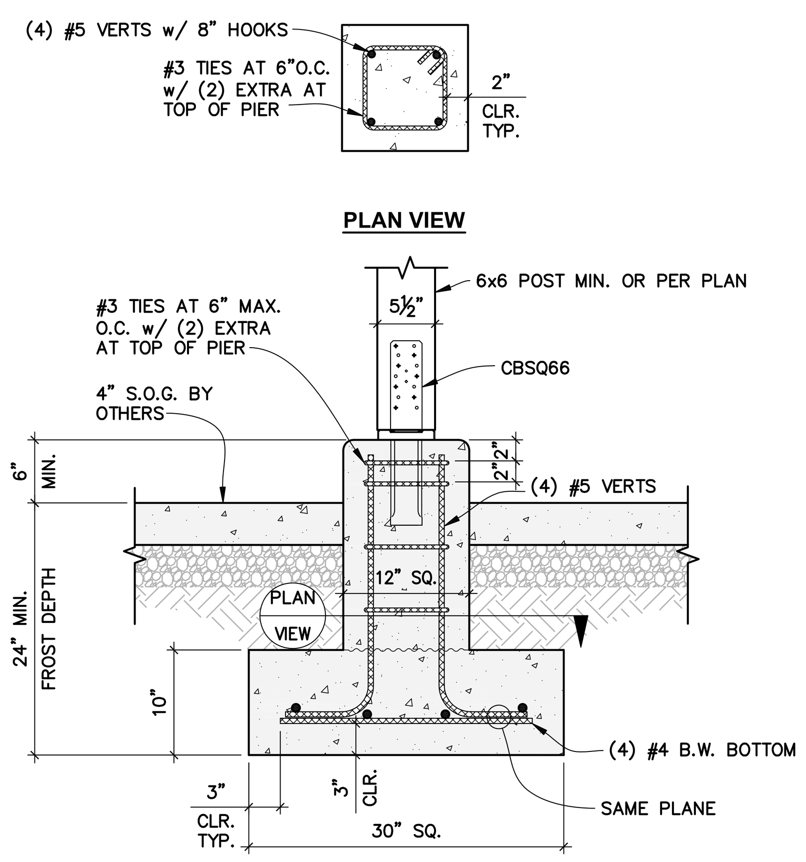
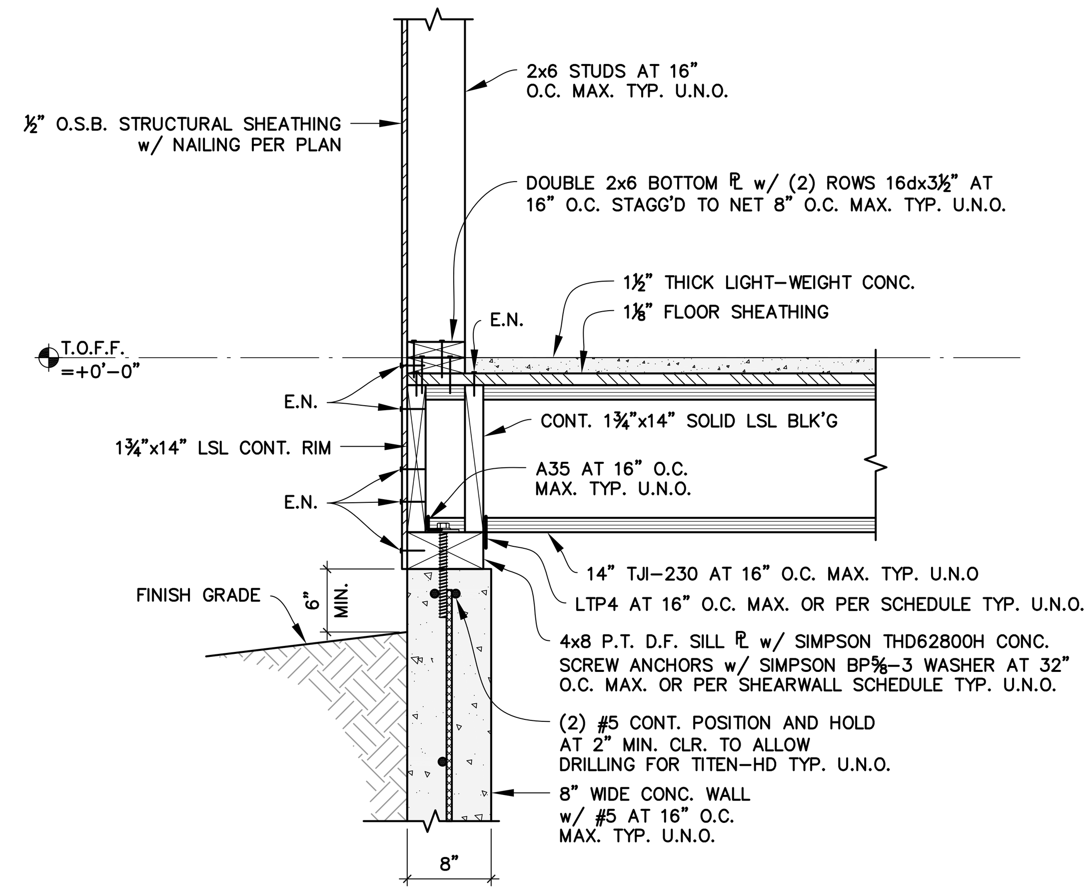
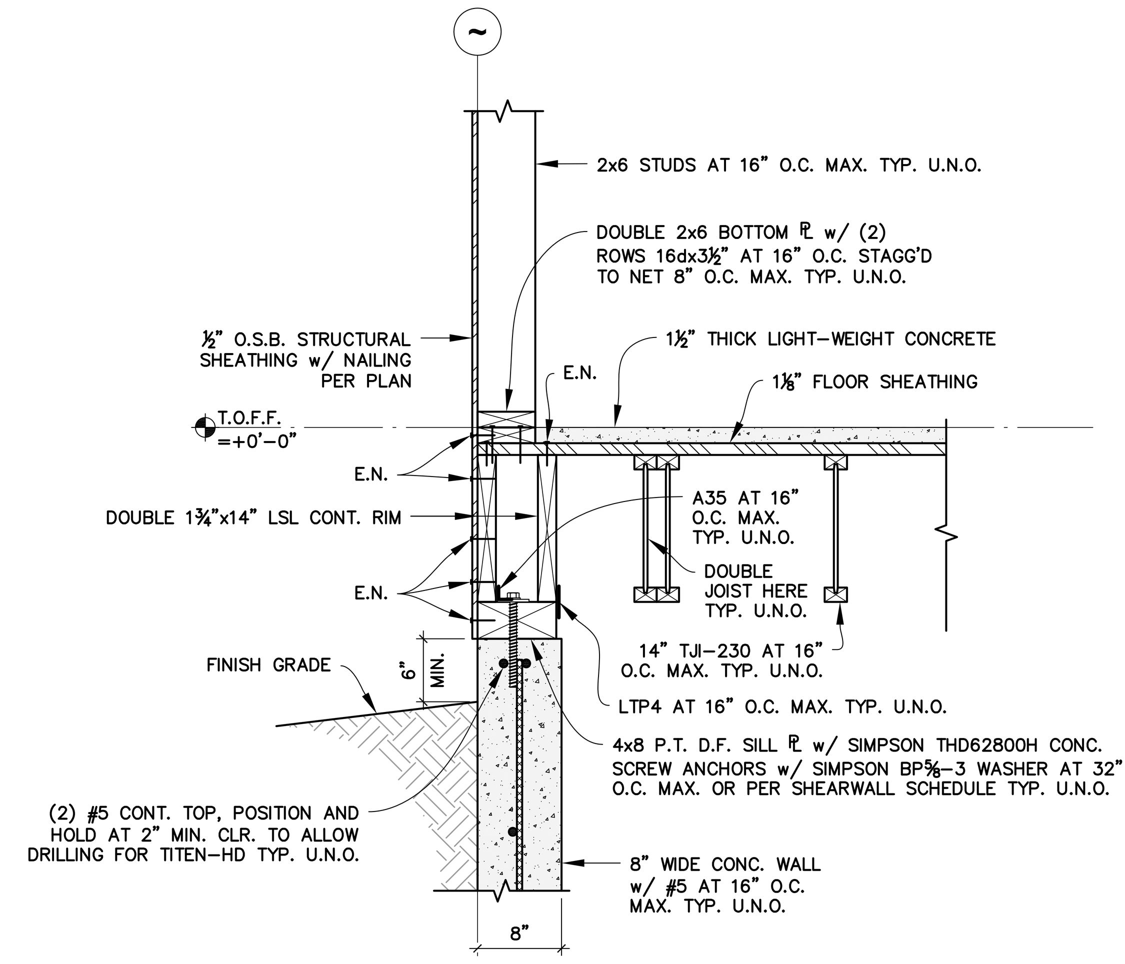


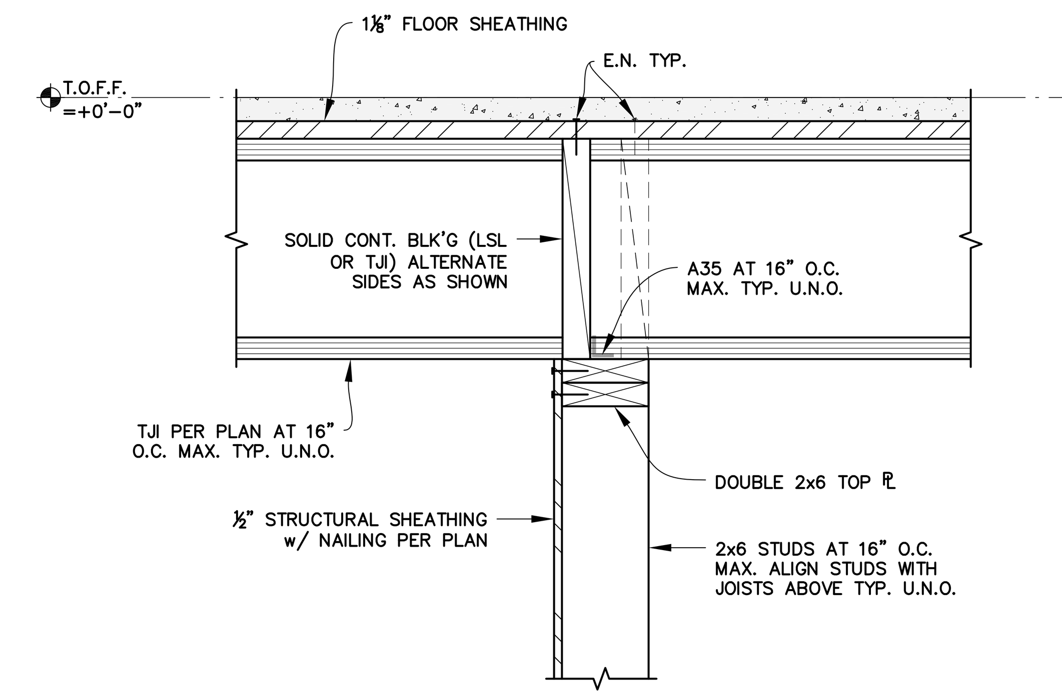
TYP. POST PEDESTAL DETAIL
SCALE: 1" = 1'-0" 1309DT14
1 S4.1



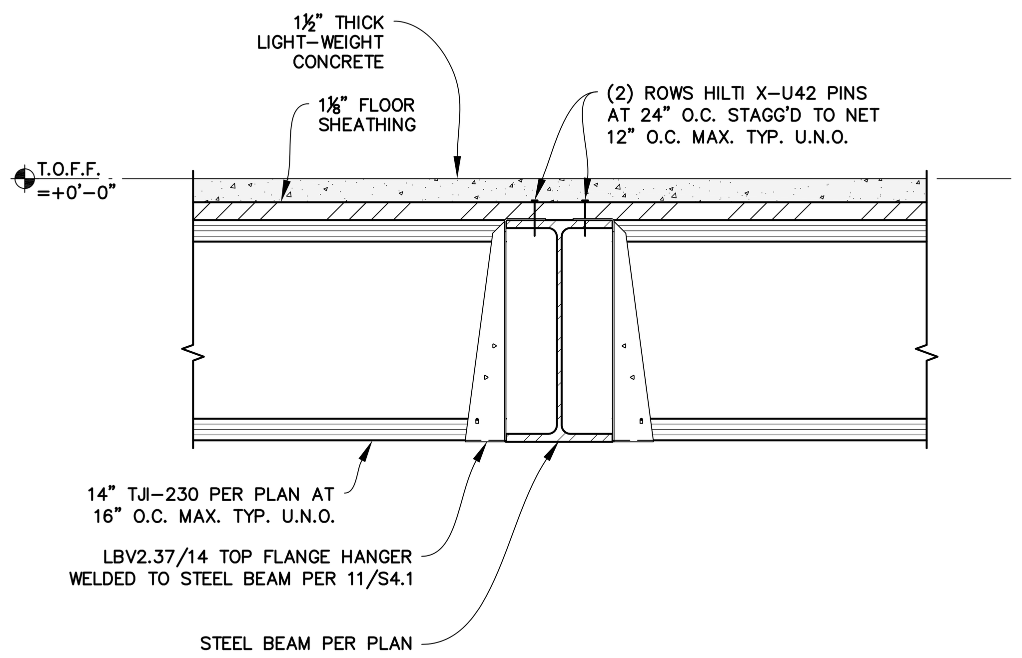
TYPICAL JOIST PERPENDICULAR TO CONC. WALL
SCALE: 1" = 1'-0" 1309DT07
2 S4.1



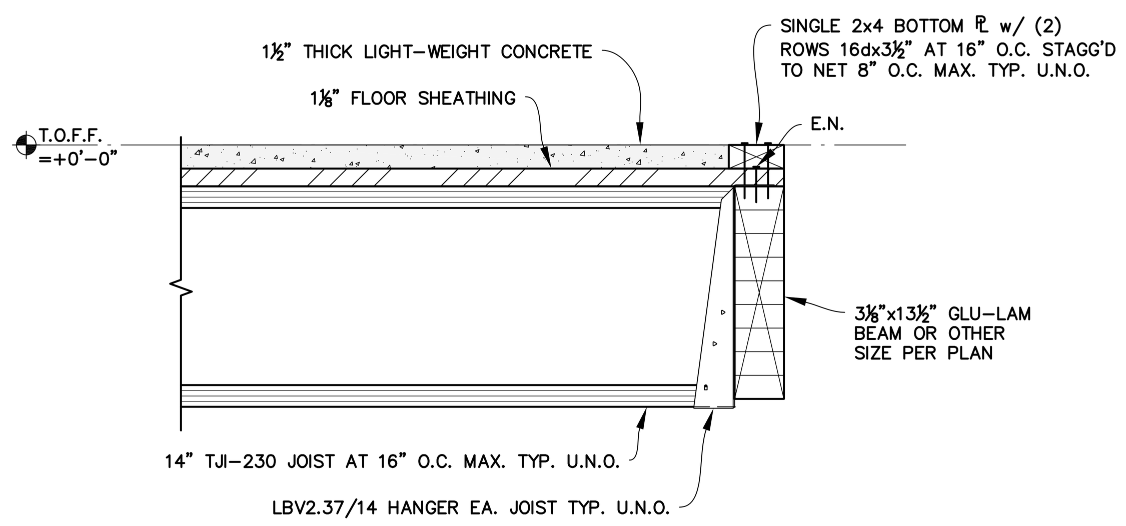
TYPICAL JOIST PARALLEL TO CONC. WALL
SCALE: 1" = 1'-0" 1309DT08
3 S4.1



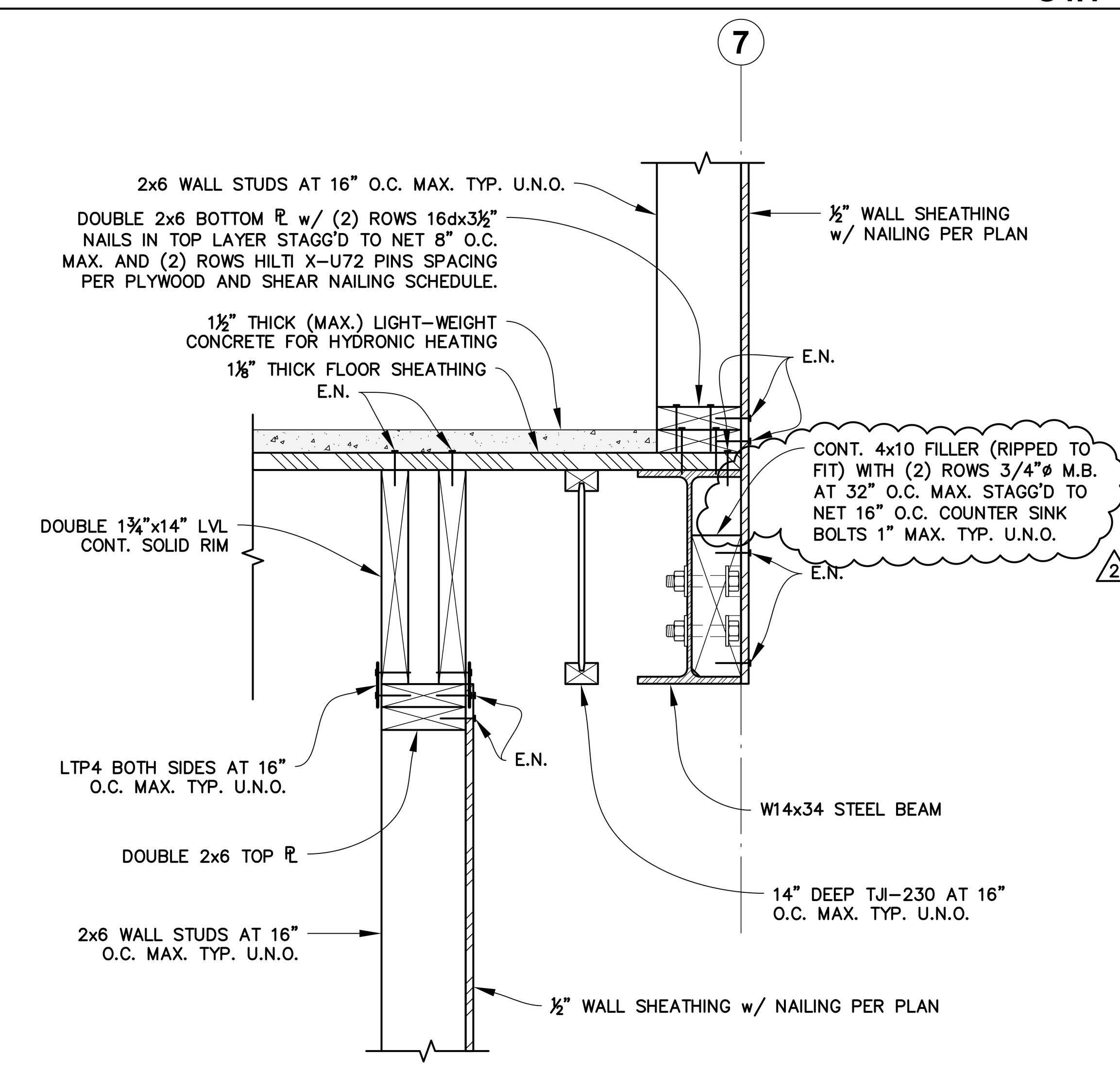
TYPICAL 2x6 STUD WALL INTERIOR BEARING/CONNECTION DETAIL
SCALE: 1½" = 1'-0" 1309DT13
4 S4.1



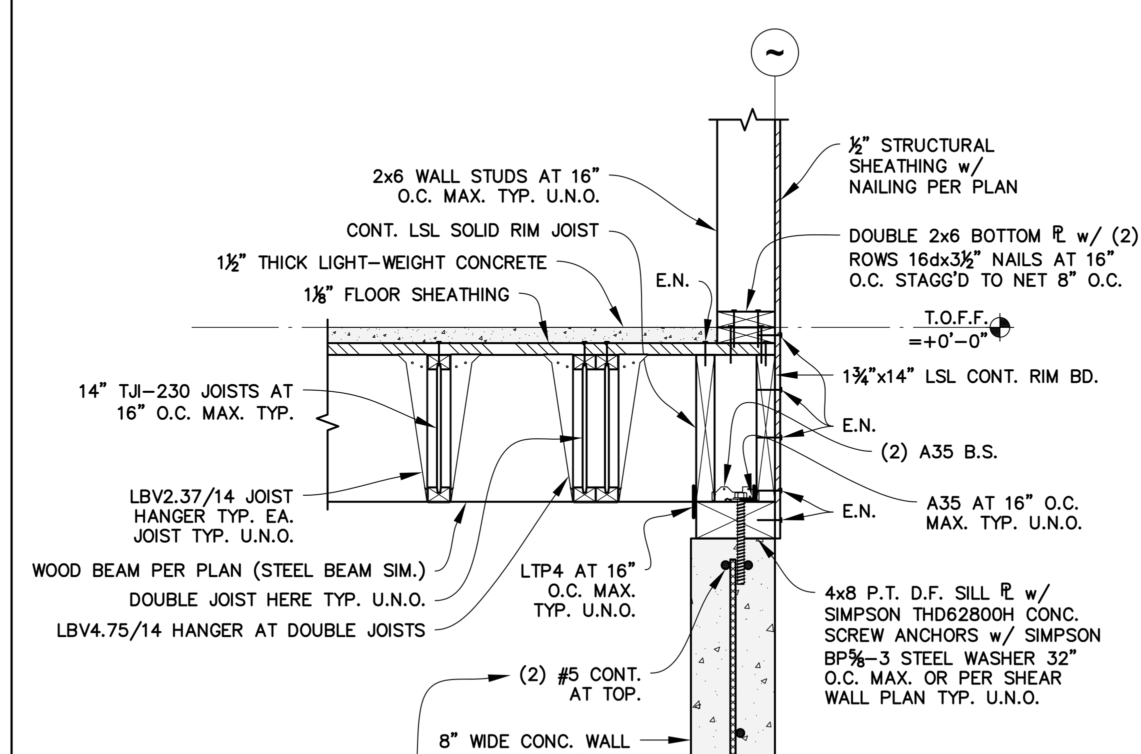
TYPICAL JOIST TO BEAM CONNECTION DETAIL
SCALE: 1½" = 1'-0" 1309DT15
5 S4.1



TYPICAL JOIST CONNECTION AT OPENINGS
SCALE: 1½" = 1'-0" 1309DT16
6 S4.1

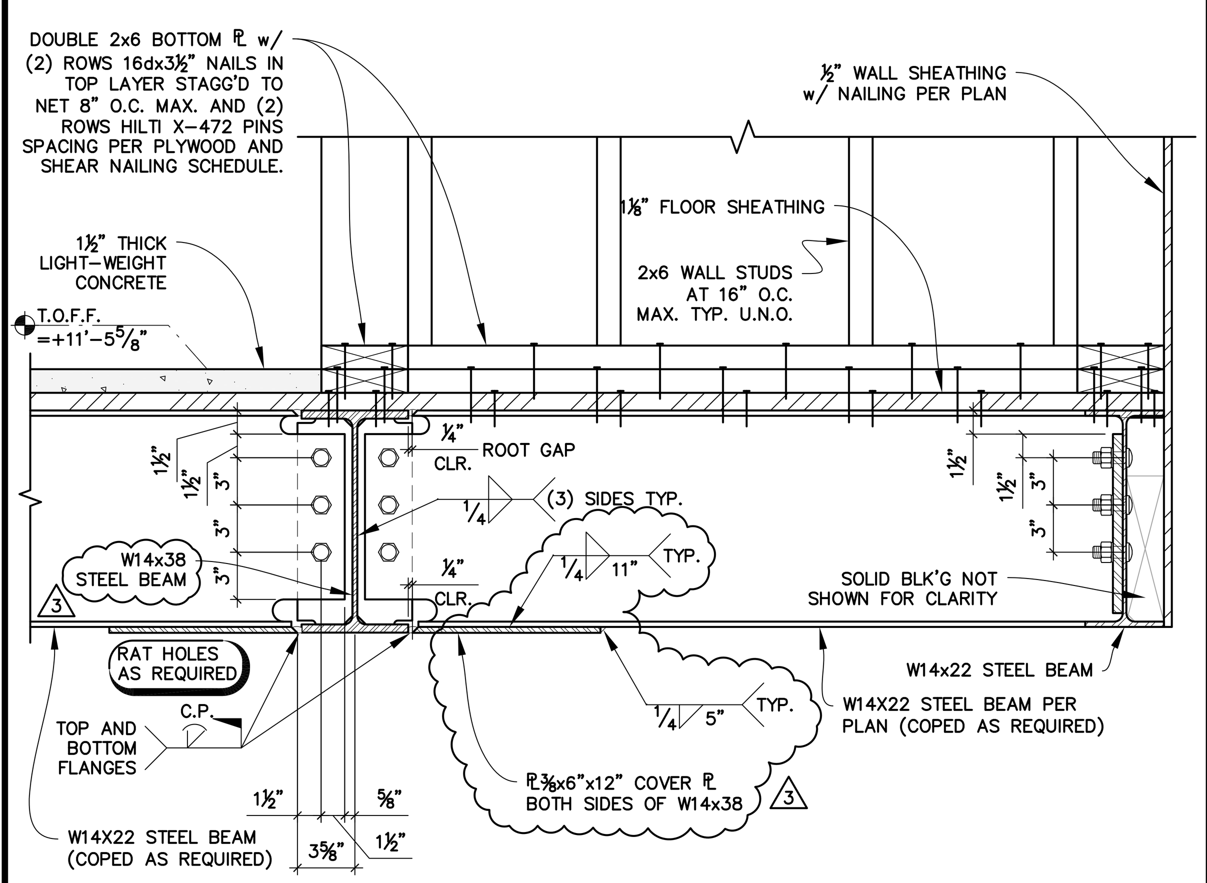


TYPICAL GARAGE FLOOR CANTILEVER CONNECTION
SCALE: 1½" = 1'-0" 1309DT05
7 S4.1



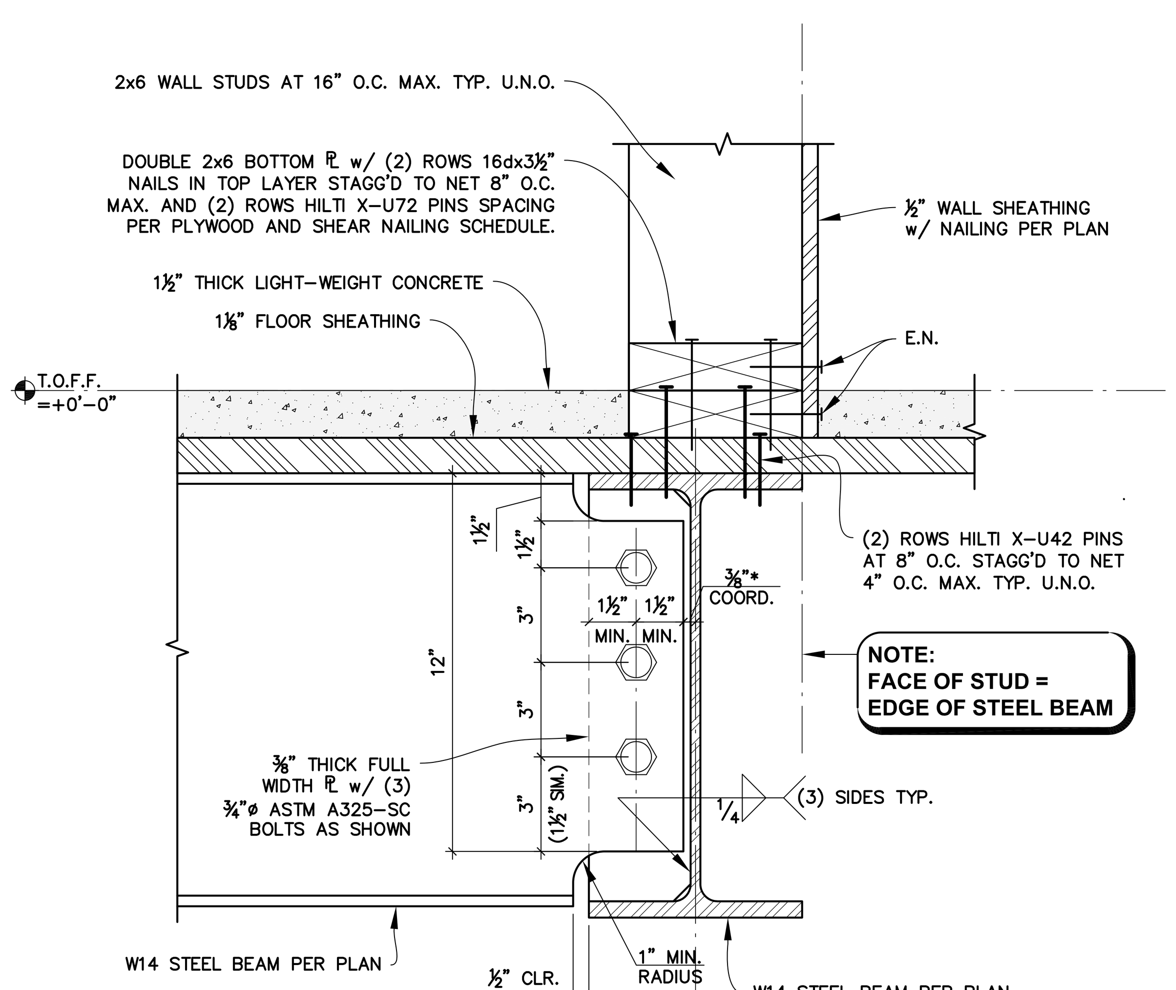
POSITION AND HOLD AT 2" MIN. CLR. TO ALLOW DRILLING FOR TITEN-HD TYP. U.N.O.

TYPICAL BEAM TO CONC. WALL CONNECTION DETAIL
SCALE: 1" = 1'-0" 1309DT19
8 S4.1



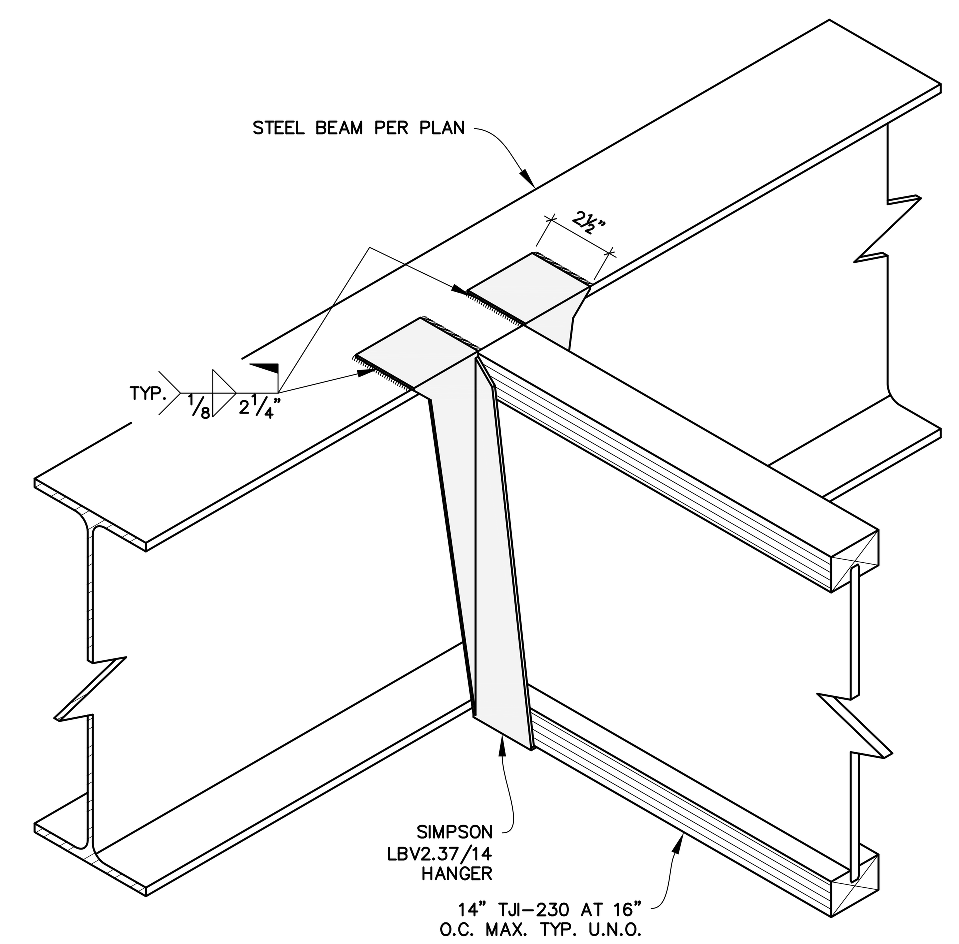
SEE DETAIL 10/S4.1 FOR SHEAR TAB INFORMATION NOT REPEATED HERE

STEEL BEAM MOMENT CONNECTION
SCALE: 1" = 1'-0" 1309DT27
9 S4.1

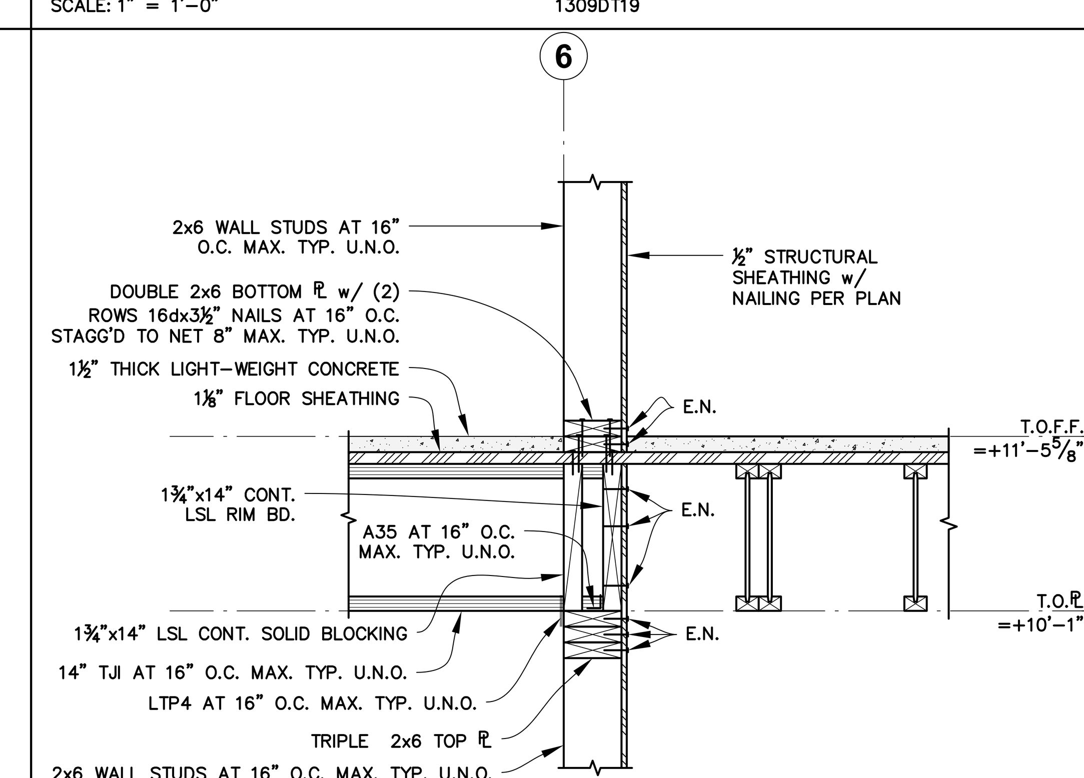


NOTE: FACE OF STUD = EDGE OF STEEL BEAM

TYPICAL STEEL BEAM CONNECTION DETAIL
SCALE: 3" = 1'-0" 1309DT28
10 S4.1



TYPICAL WELDED HANGER CONNECTION DETAIL
SCALE: ISO(3" = 1'-0") 1309DT40
11 S4.1



SHEAR CONNECTION AT GARAGE WALL
SCALE: 1" = 1'-0" 1309DT45
12 S4.1

NOTE:
TYPICAL DETAILS ARE NOT USUALLY CUT OR REFERENCED. THEY ARE INTENDED TO APPLY IN ALL SUCH SIMILAR CONDITIONS, WHETHER REFERENCED OR NOT.

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ISSUED FOR BUILDING DEPARTMENT PLAN CHECK
May 27, 2009

PROFESSIONAL ENGINEER STATE OF NEVADA — KIRK N. ELLIS — No. 4398 — STRUCTURAL — 5/27/2009

KNE
KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST. #425 RENO, NV 89501
PHONE: (775)322-7723 FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:
1 08/31/2009 ARCH'L COORD.
2 09/21/2009 BUILDING DEPT.
3 09/10/2009 BUILDING DEPT.

DATE:	05/27/2009
SCALE:	AS SHOWN
DRAWN:	BLW
DESIGNED:	TRH/KNE
CHECKED:	KNE
KNE JOB NO.	1309

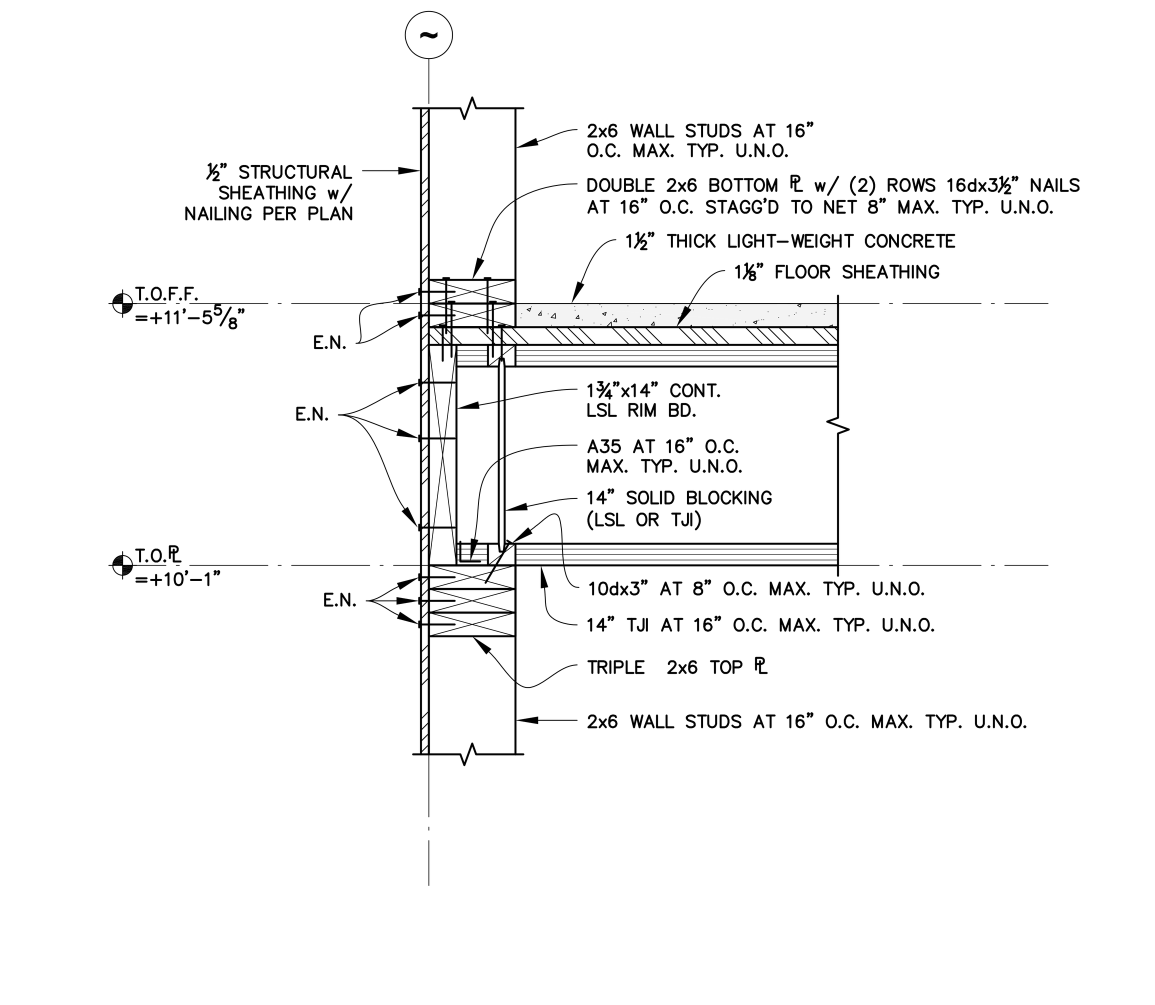
SHEET TITLE: STRUCTURAL FLOOR FRAMING DETAILS

SHEET NO.
S4.1
OF 15 SHEETS

ATTIC VENTILATION

ARCHITECT SHALL PROVIDE POSITIVE ATTIC VENTILATION PER THE 2006 I.B.C. AND APPLICABLE WASHOE COUNTY CODES

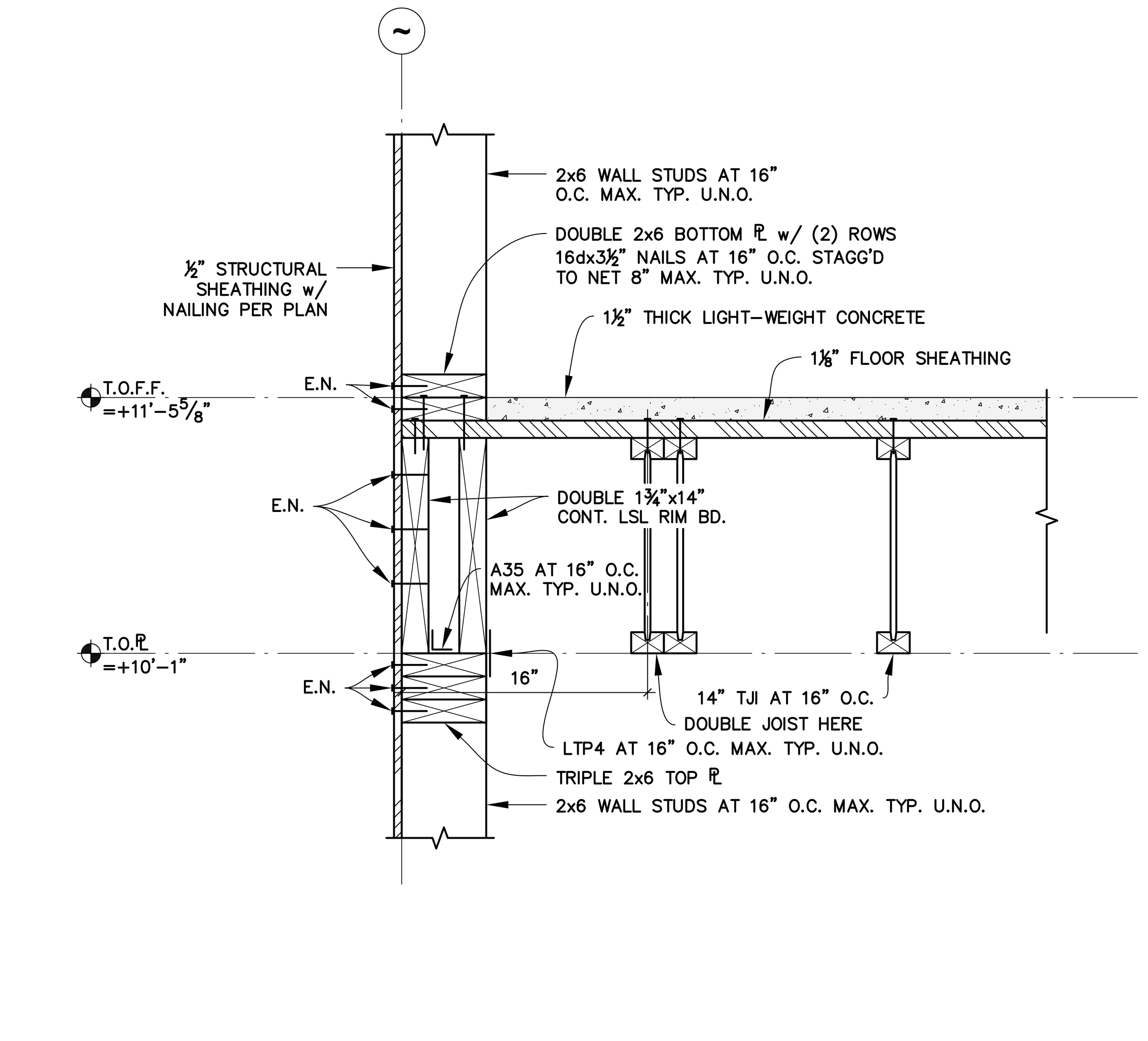
COORDINATE ALL DRAINAGE AND FLASHING WITH ARCHITECTURAL PLANS



TYPICAL SECOND FLOOR CONNECTION AT JOISTS PERPENDICULAR TO WALL

1 S4.2

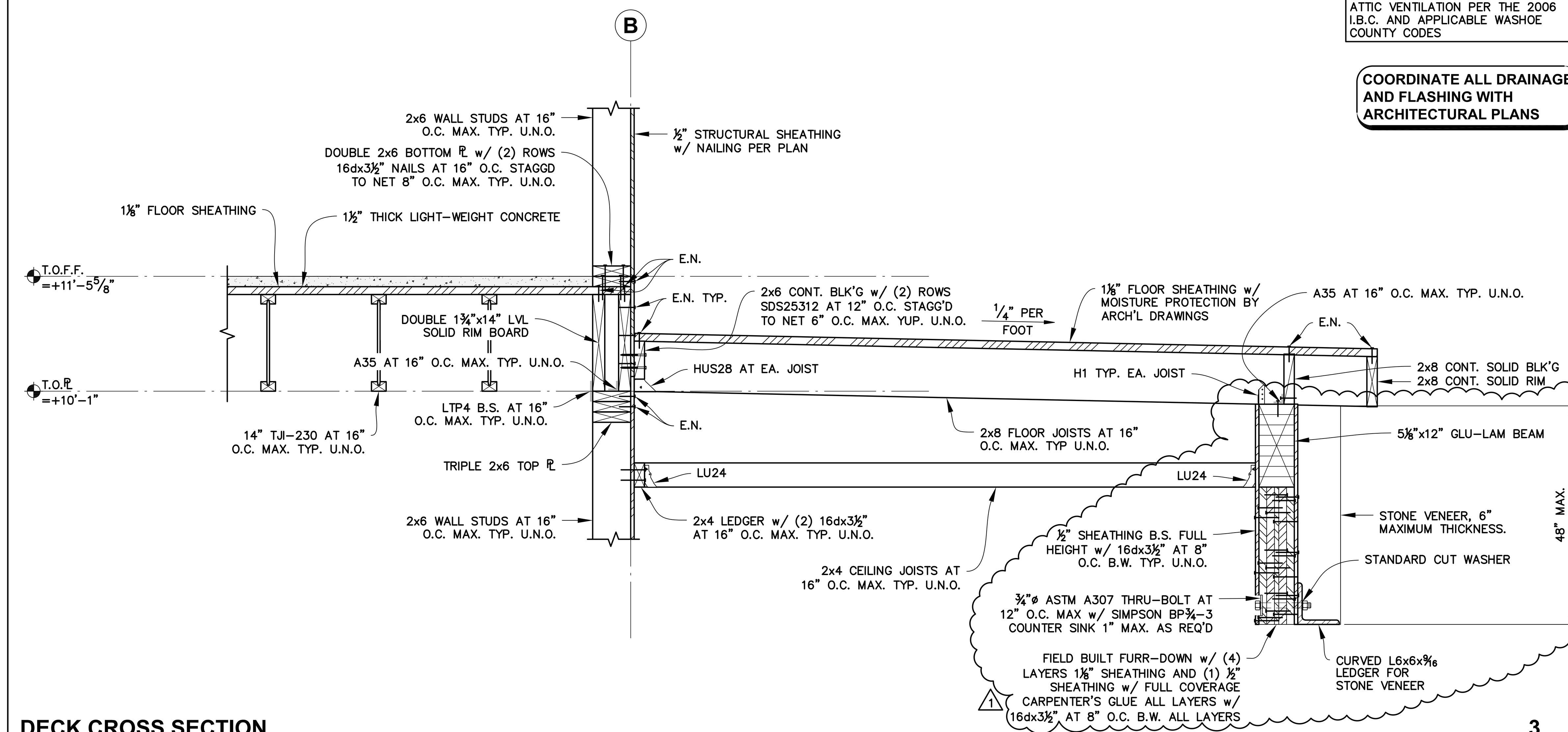
SCALE: 1½" = 1'-0" 1309DT20



TYPICAL SECOND FLOOR CONNECTION AT JOISTS PARALLEL TO WALL

2 S4.2

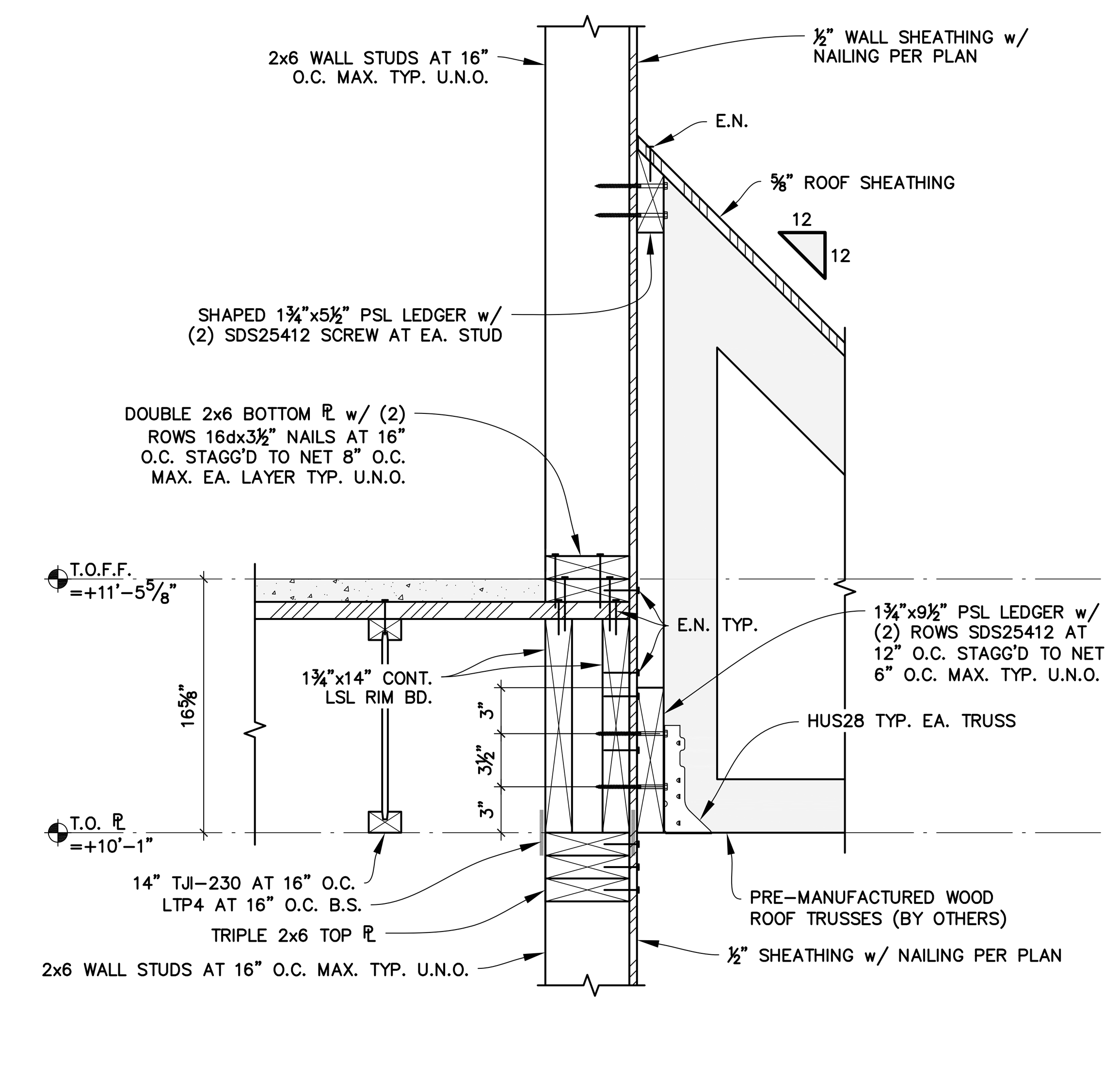
SCALE: 1½" = 1'-0" 1309DT21



DECK CROSS SECTION

3 S4.2

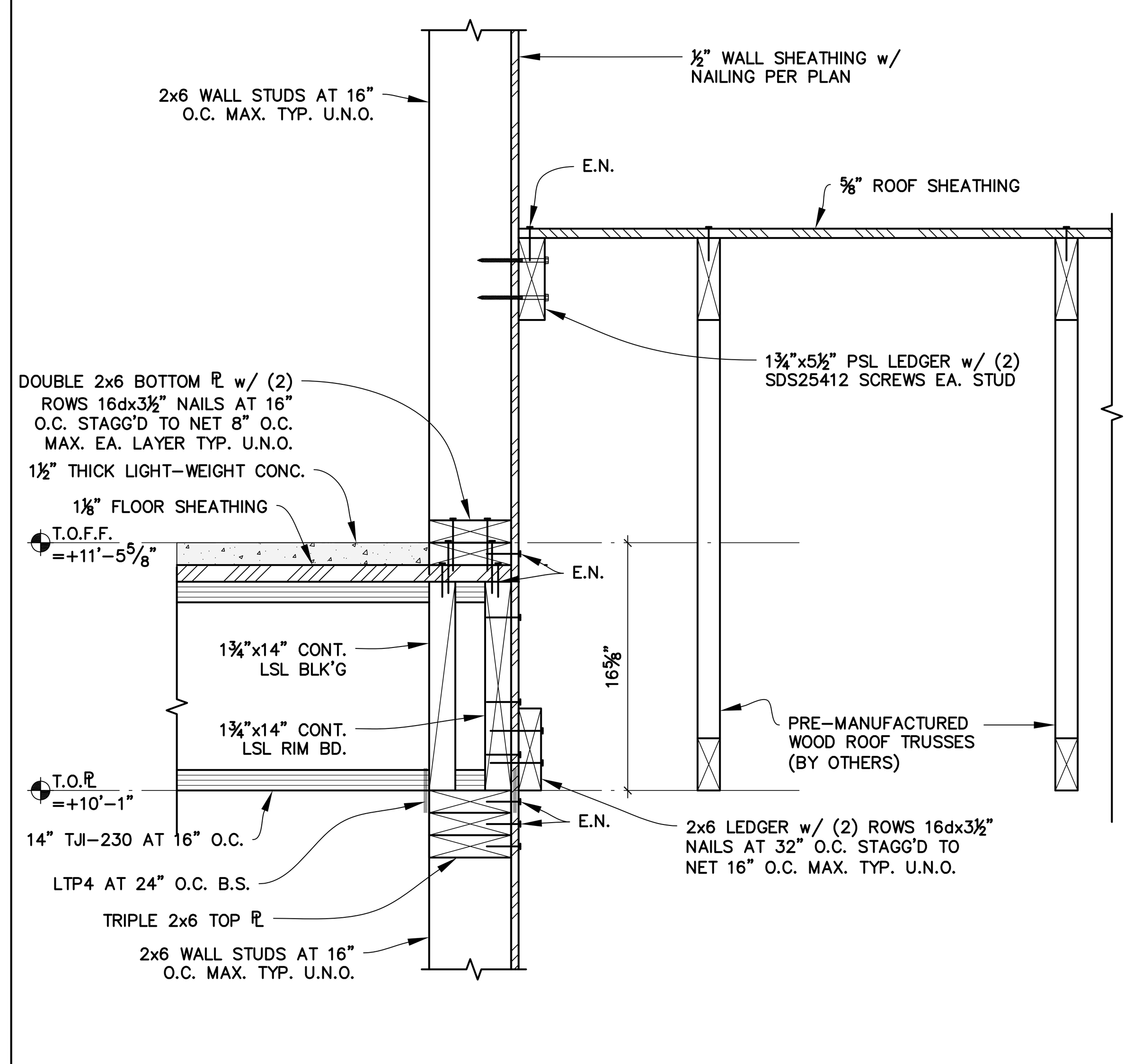
SCALE: 1" = 1'-0" 1309DT25



ROOF TRUSS TO FLOOR CONNECTION DETAIL

4 S4.2

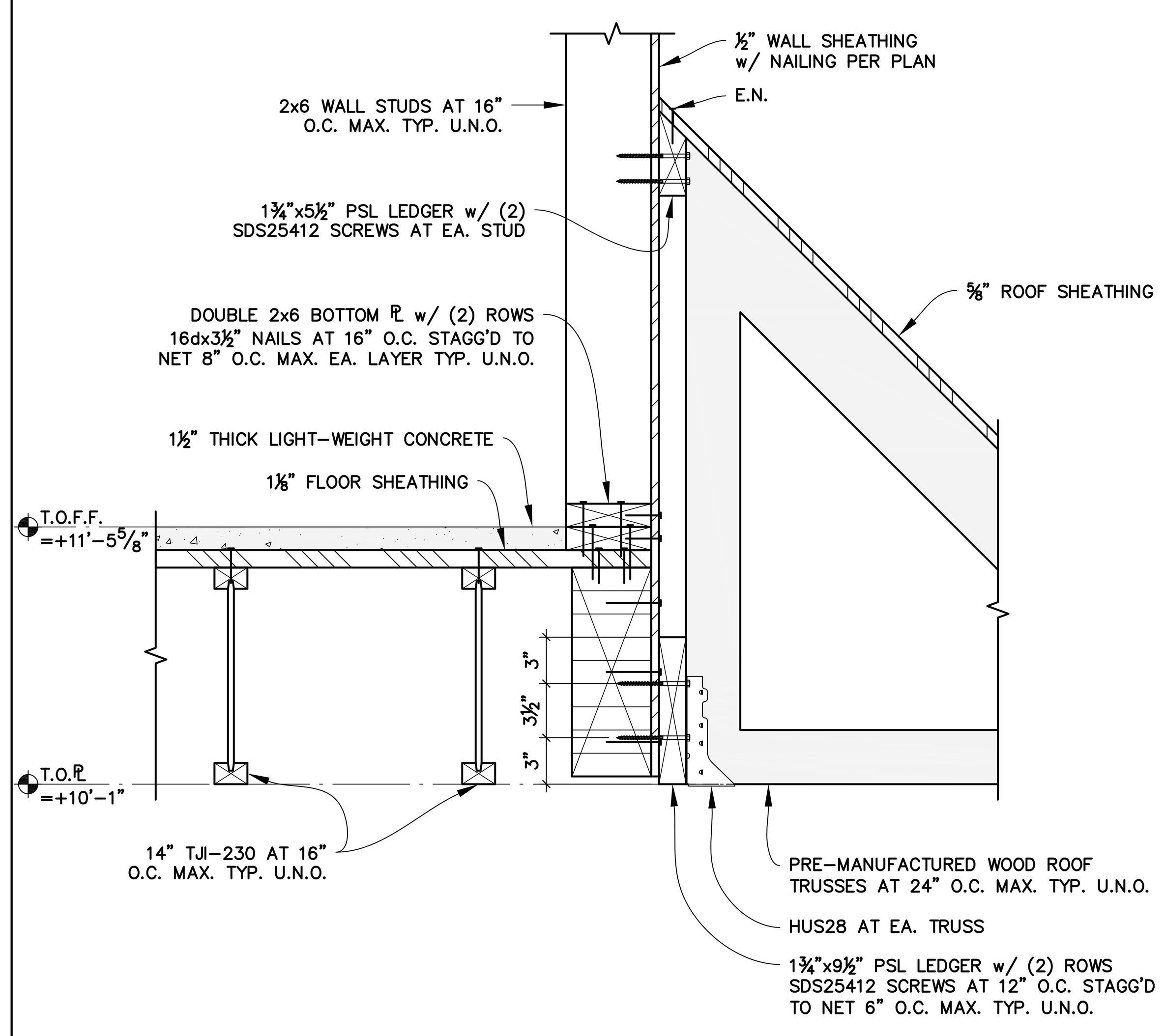
SCALE: 1½" = 1'-0" 1309DT43



ROOF TRUSS TO FLOOR CONNECTION DETAIL

5 S4.2

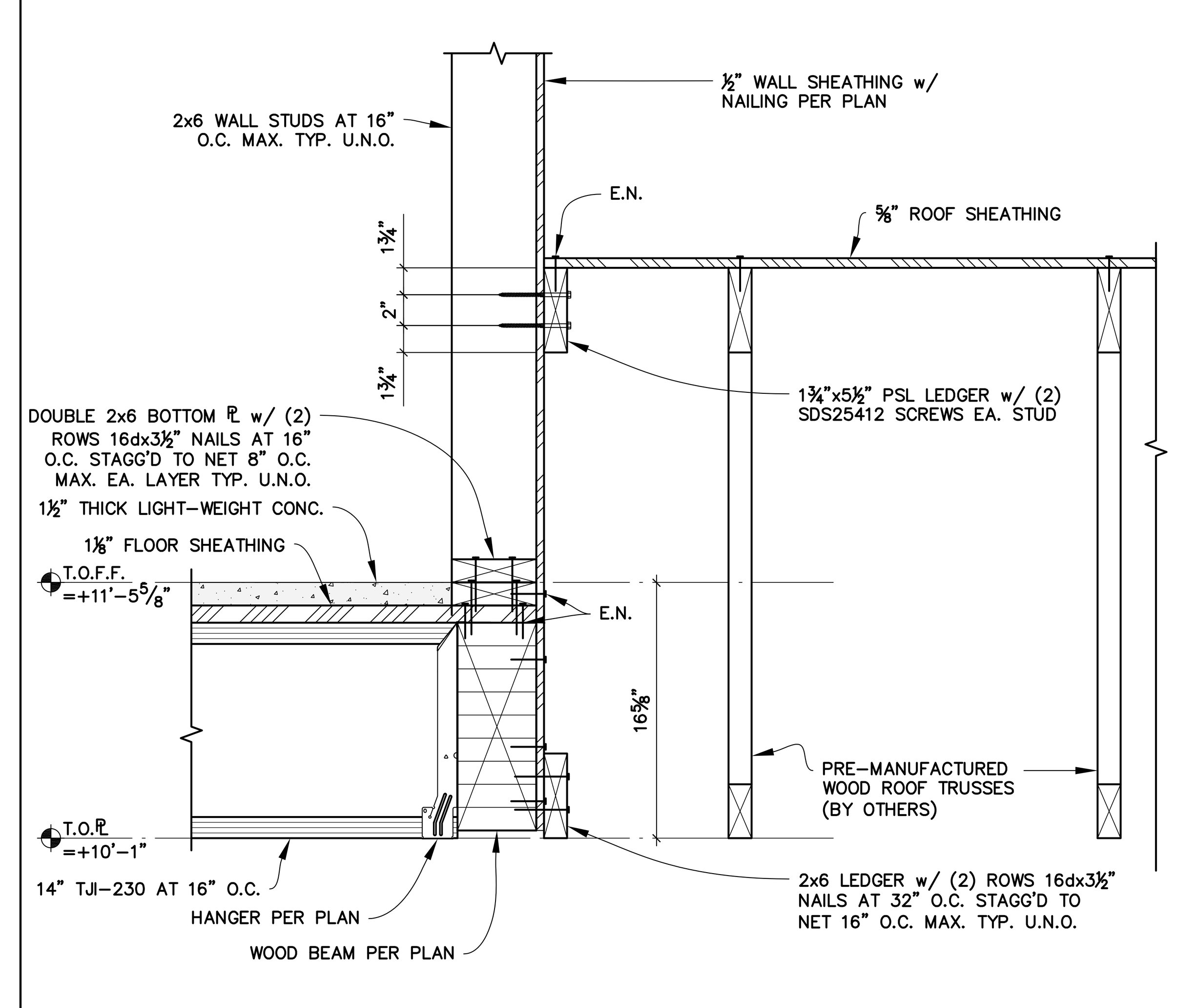
SCALE: 1½" = 1'-0" 1309DT42



TYP. FLOOR BEAM TO TRUSS CONNECTION

6 S4.2

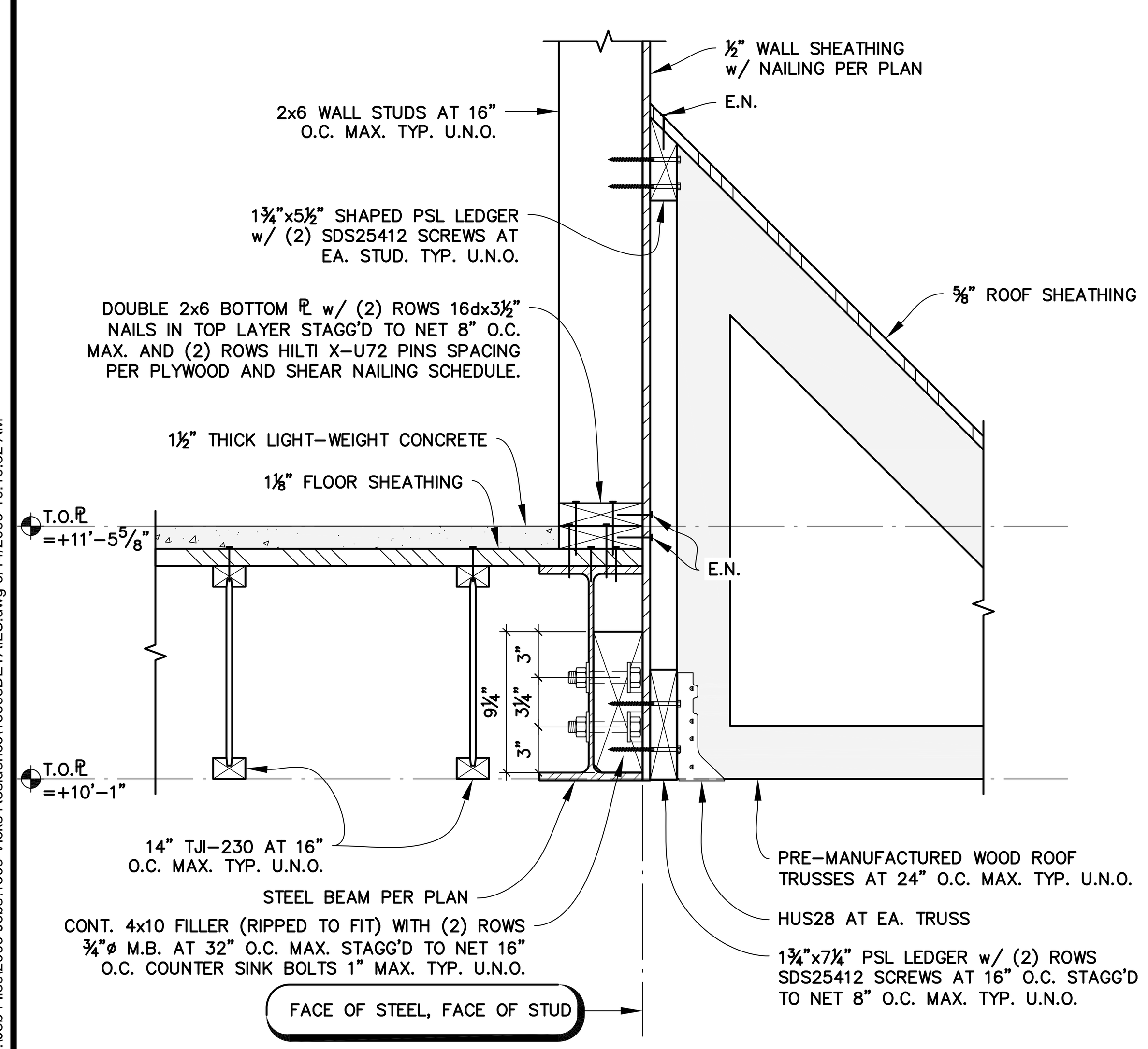
SCALE: 1½" = 1'-0" 1309DT11



ROOF TRUSS TO FLOOR CONNECTION DETAIL

7 S4.2

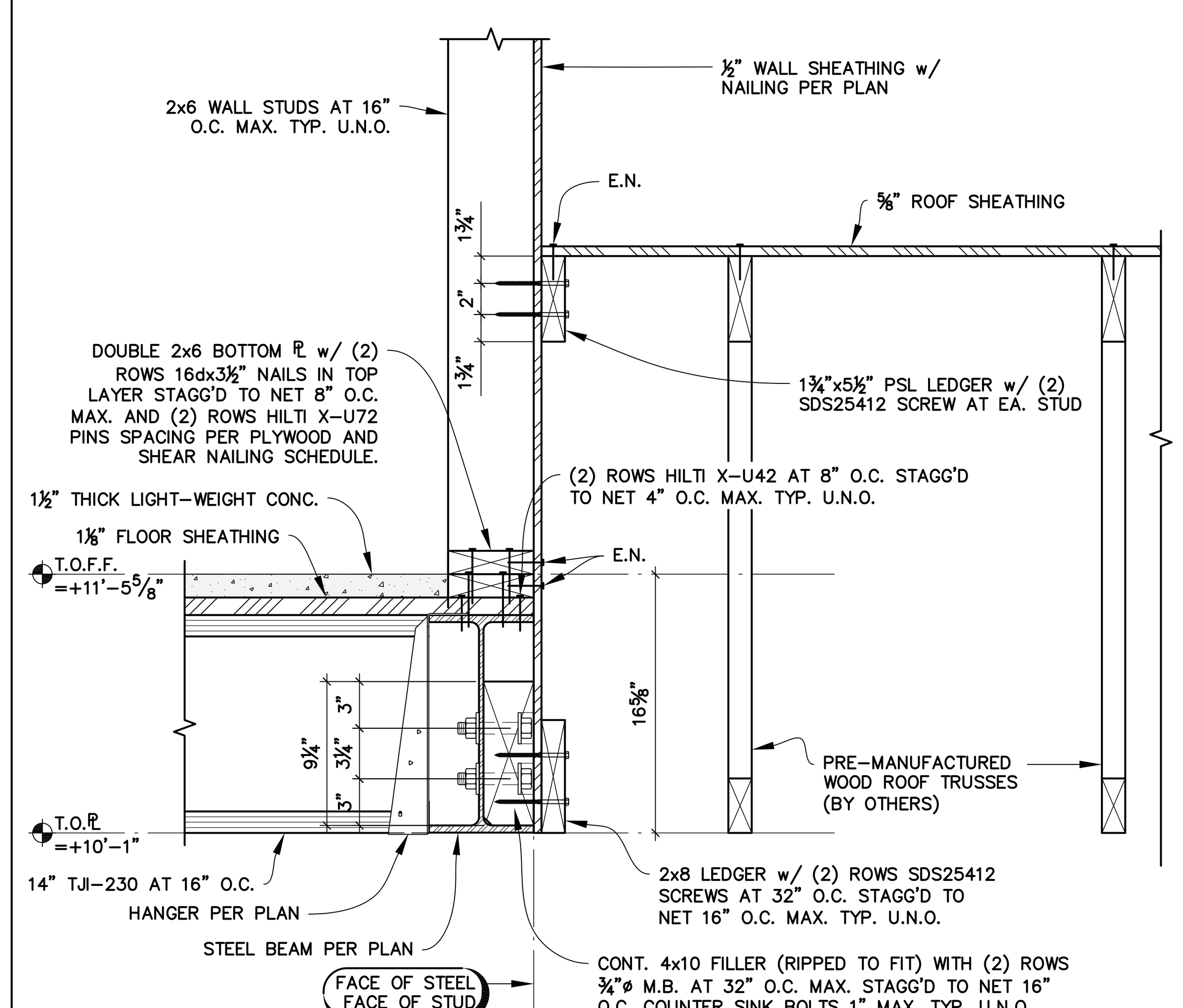
SCALE: 1½" = 1'-0" 1309DT22



TYP. FLOOR BEAM TO TRUSS CONNECTION

8 S4.2

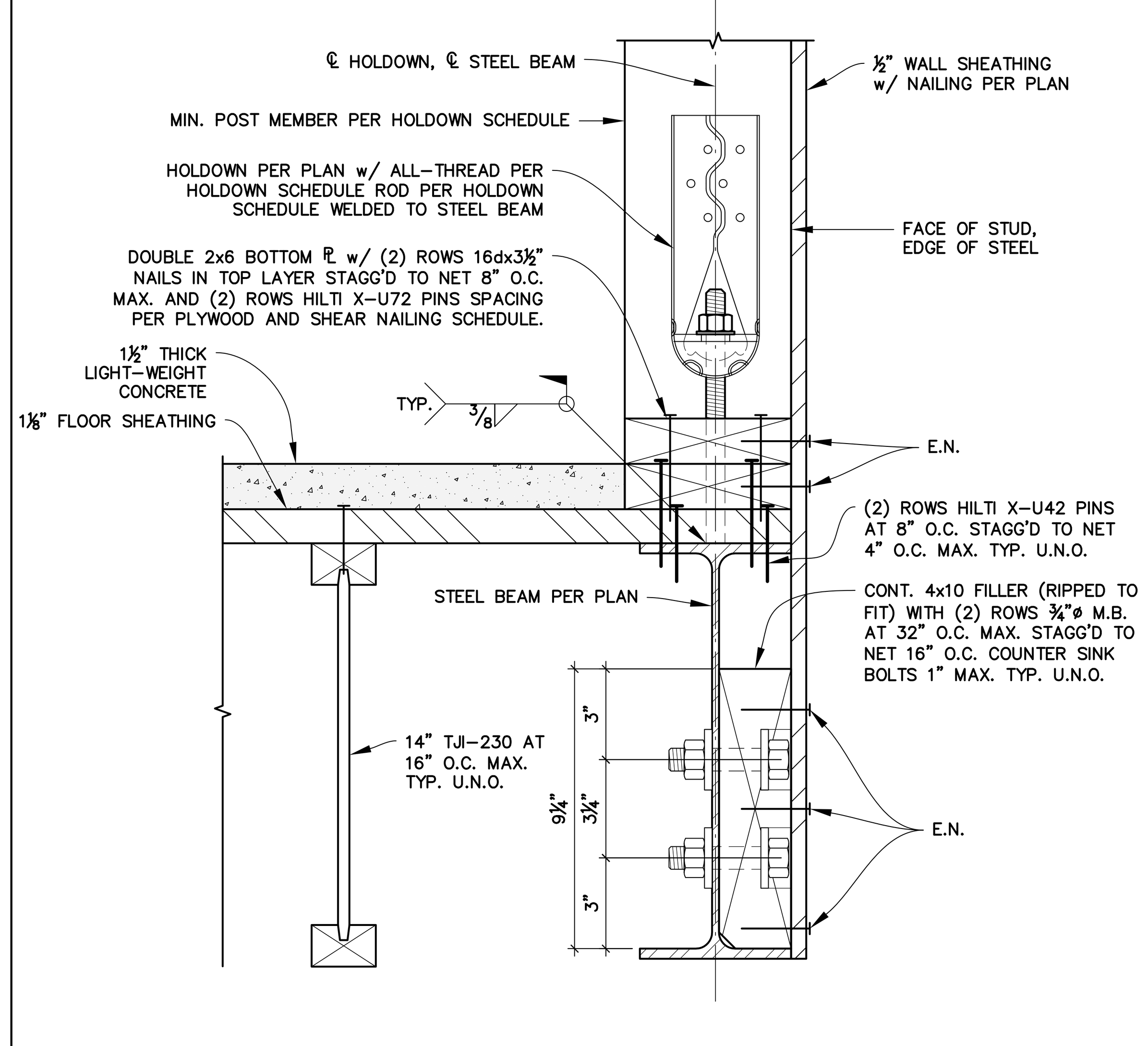
SCALE: 1½" = 1'-0" 1309DT23



ROOF TRUSS TO FLOOR CONNECTION DETAIL

9 S4.2

SCALE: 1½" = 1'-0" 1309FT24



TYPICAL HOLDOWN AT STEEL BEAM CONNECTION DETAIL

10 S4.2

SCALE: NO SCALE 1309DT41

NOTE: EXTERIOR FACE OF STUD (F.O.S.) SETS EDGE OF STEEL BEAMS PER DETAILS. COORDINATE AS REQUIRED. SUBMIT STRUCTURAL STEEL SHOP DRAWINGS.

NOTE:
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May 27, 2009

PROFESSIONAL ENGINEER STATE OF NEVADA — KIRK N. ELLIS — No. 4398 — STRUCTURAL — 5/27/2009

KNE
KIRK N. ELLIS & ASSOCIATES
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PHONE: (775)322-7723 FAX: (775)322-5372
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DATE:	05/27/2009
SCALE:	AS SHOWN
DRAWN:	BLW
DESIGNED:	TRH/KNE
CHECKED:	KNE
KNE JOB NO.	1309

SHEET TITLE: STRUCTURAL FLOOR TO FLOOR DETAILS

SHEET NO. S4.2
OF 15 SHEETS

ATTIC VENTILATION

ARCHITECT SHALL PROVIDE POSITIVE ATTIC VENTILATION PER THE 2006 I.B.C. AND APPLICABLE WASHOE COUNTY CODES

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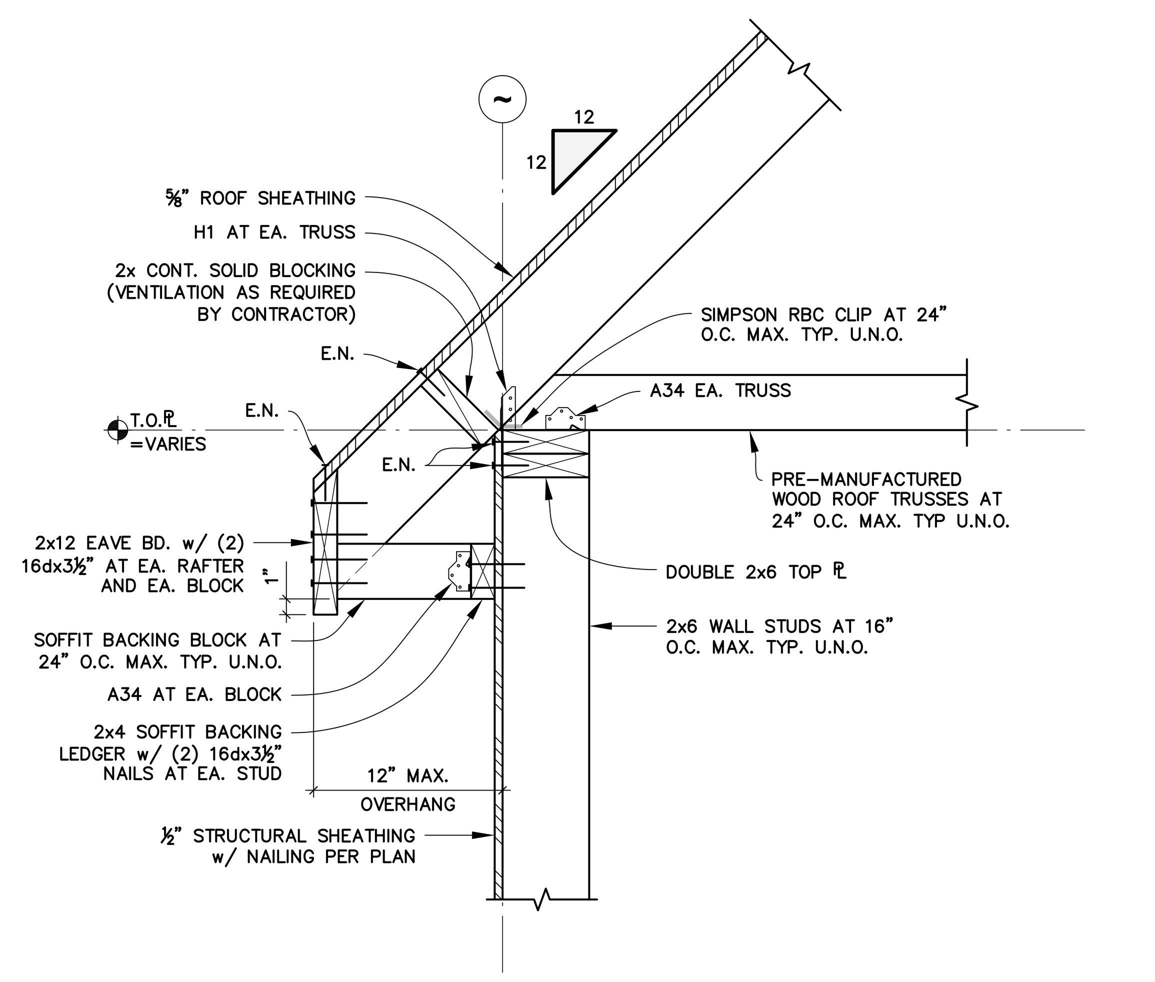
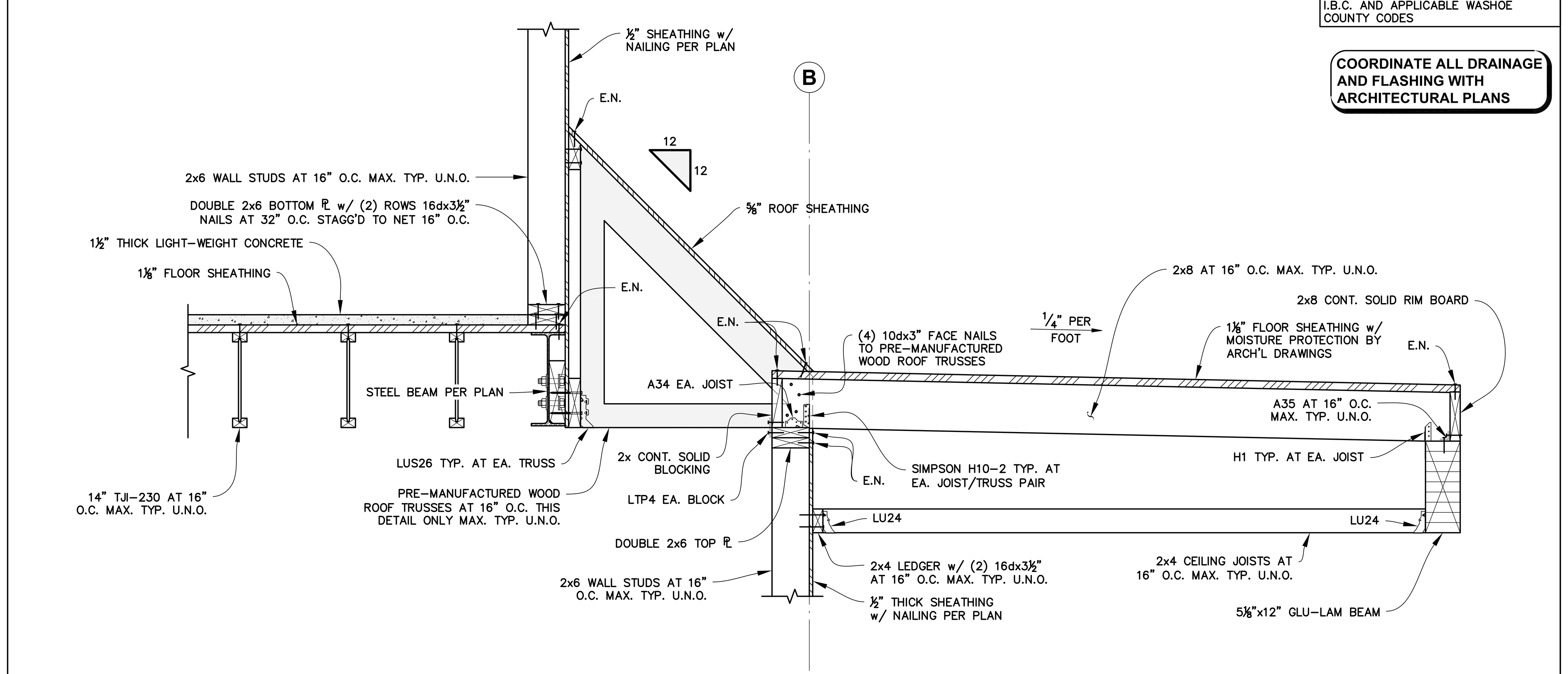


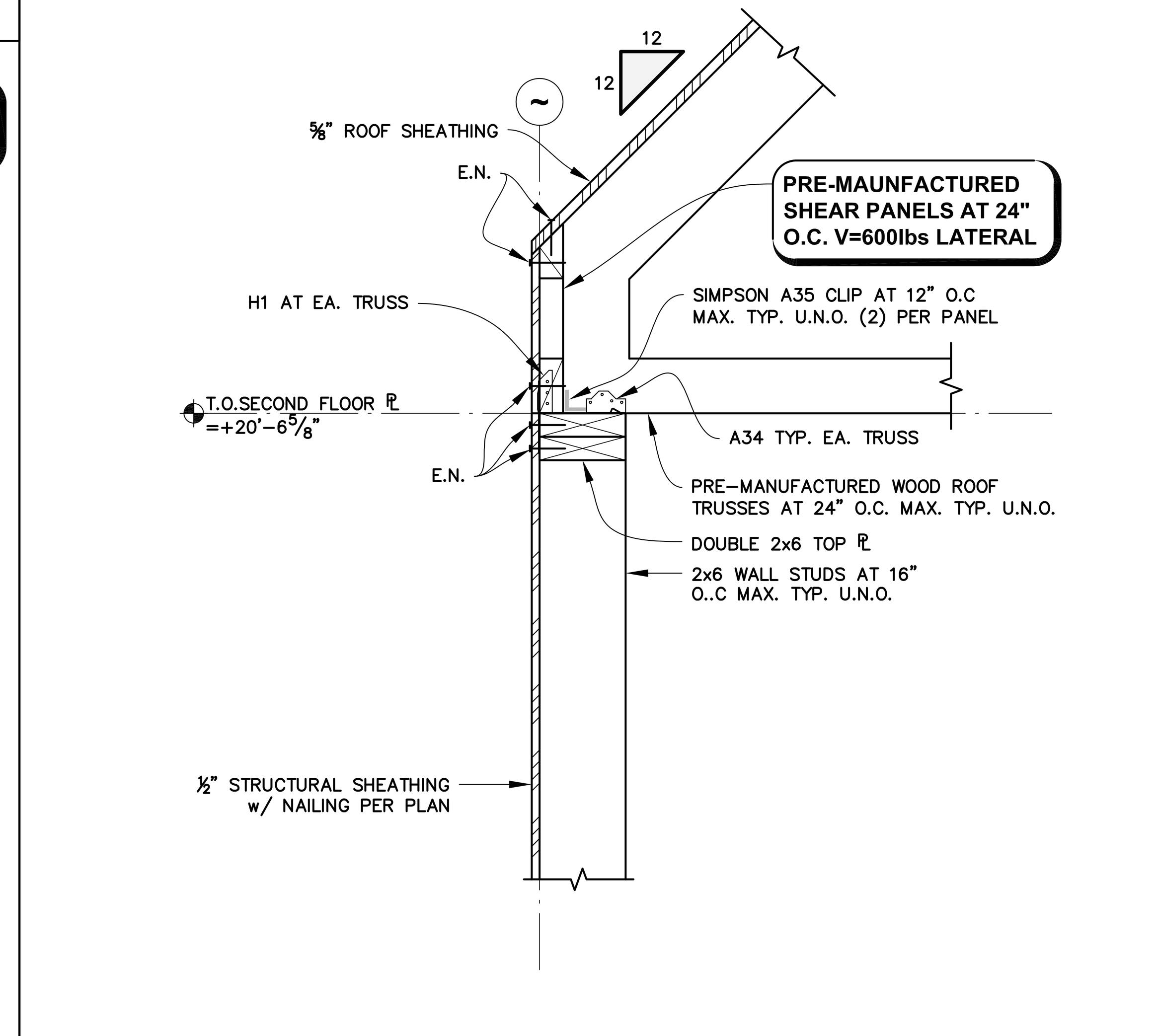
TYP. ROOF TRUSS CONN. AT EXTERIOR BEARING WALL — 1 / S4.3

SCALE: 1½" = 1'-0" 1309DT29



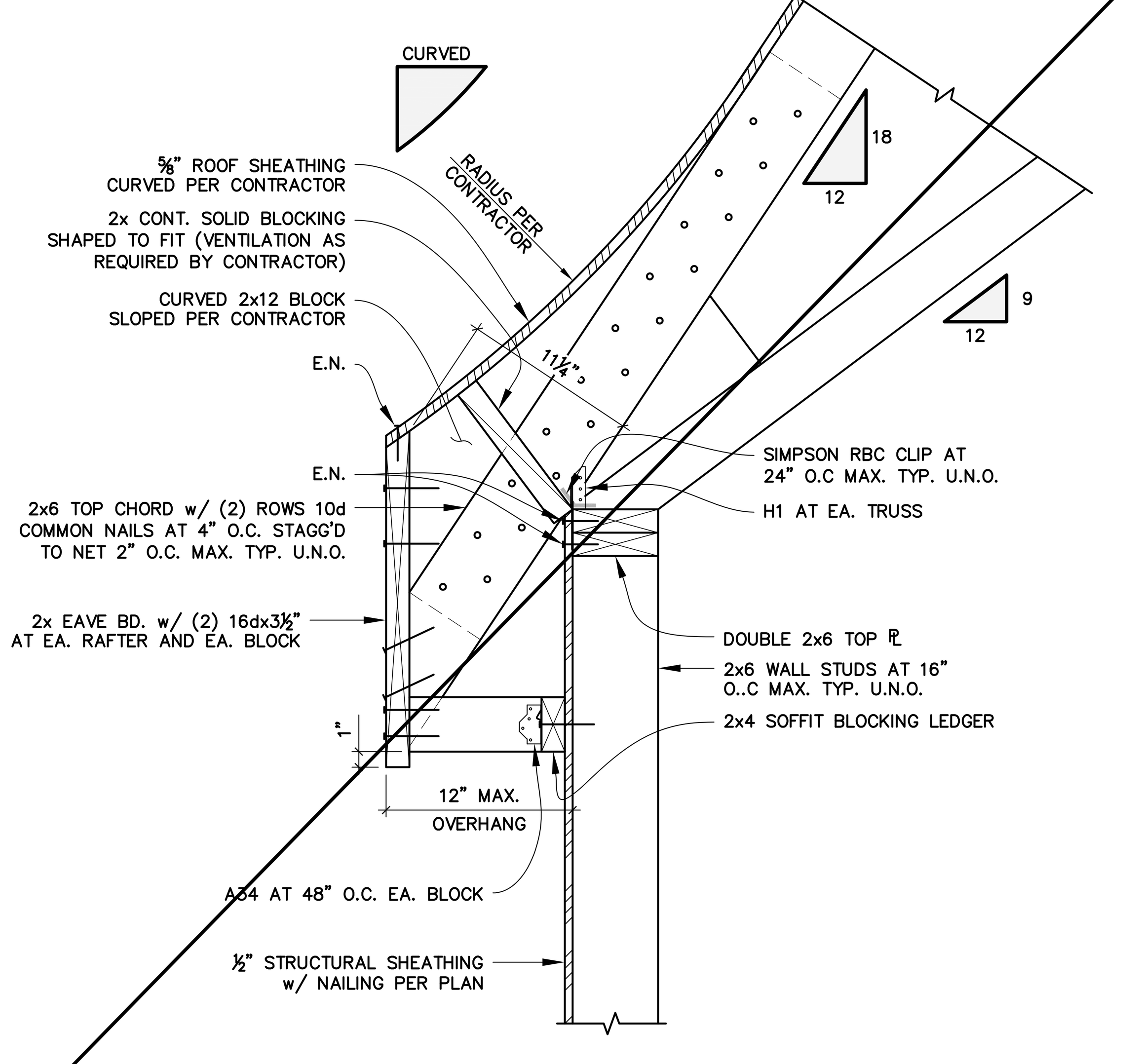
TYP. DECK CONNECTION AT TOP PLATE — 2 / S4.3

SCALE: 1" = 1'-0" 1309DT26



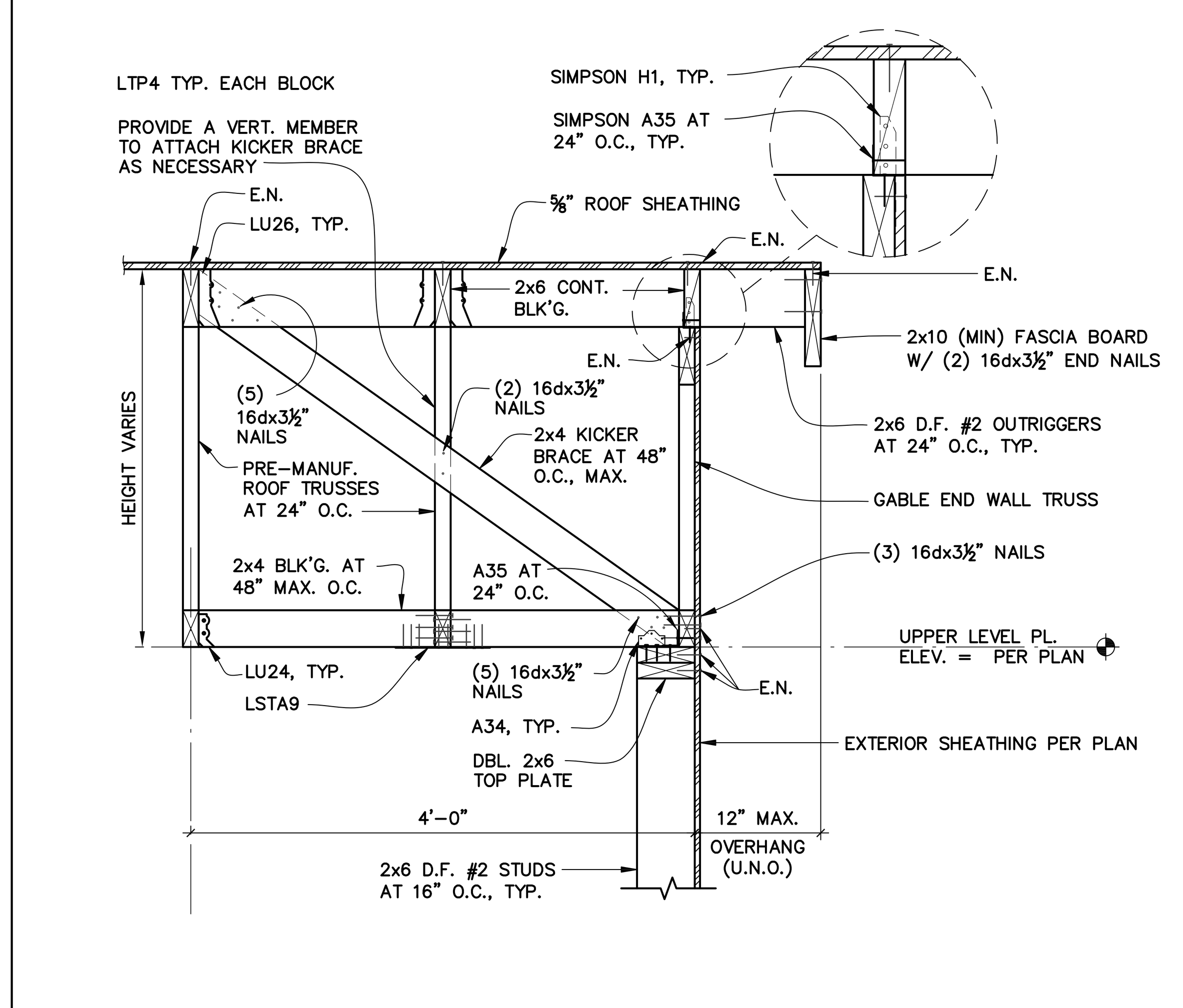
TYPICAL ROOF TRUSS CONNECTION AT EXTERIOR WALL — 3 / S4.3

SCALE: 1½" = 1'-0" 1309DT32



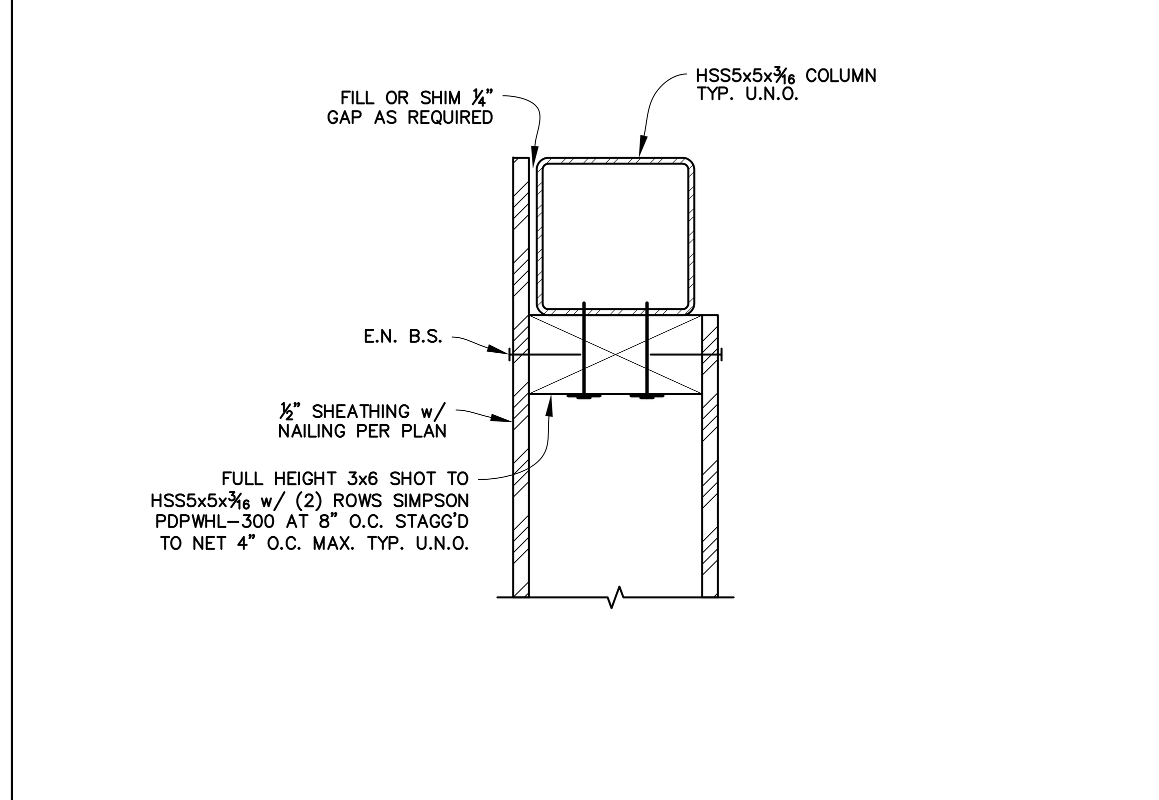
TYPICAL CURVED ROOF EAVE DETAIL — 4 / S4.3

SCALE: 1½" = 1'-0" 1309DT34



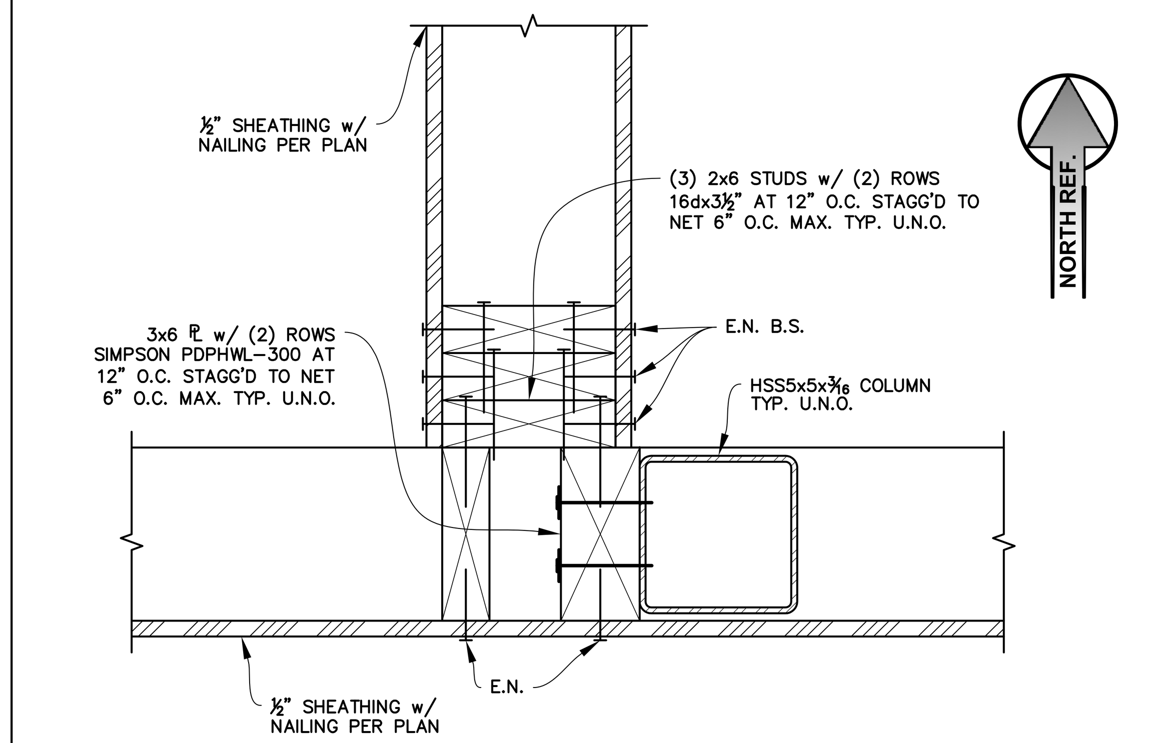
TYP. SECTION AT GABLE END WALL TRUSS — 5 / S4.3

SCALE: 1" = 1'-0" 1309TYP06512



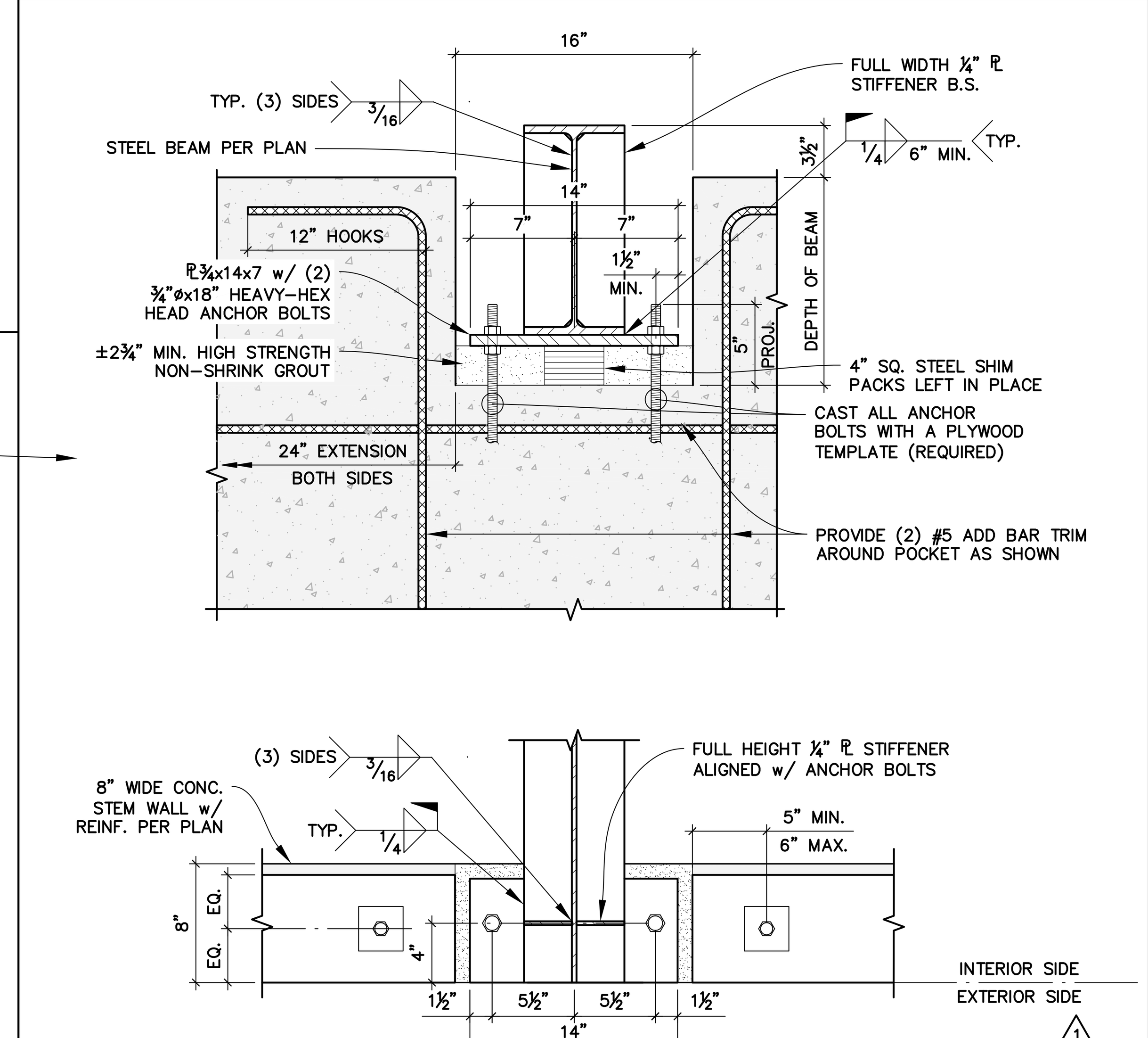
TYPICAL WOOD WALL TO STEEL COLUMN DETAIL — 6 / S4.3

SCALE: 3" = 1'-0" 1309DT50



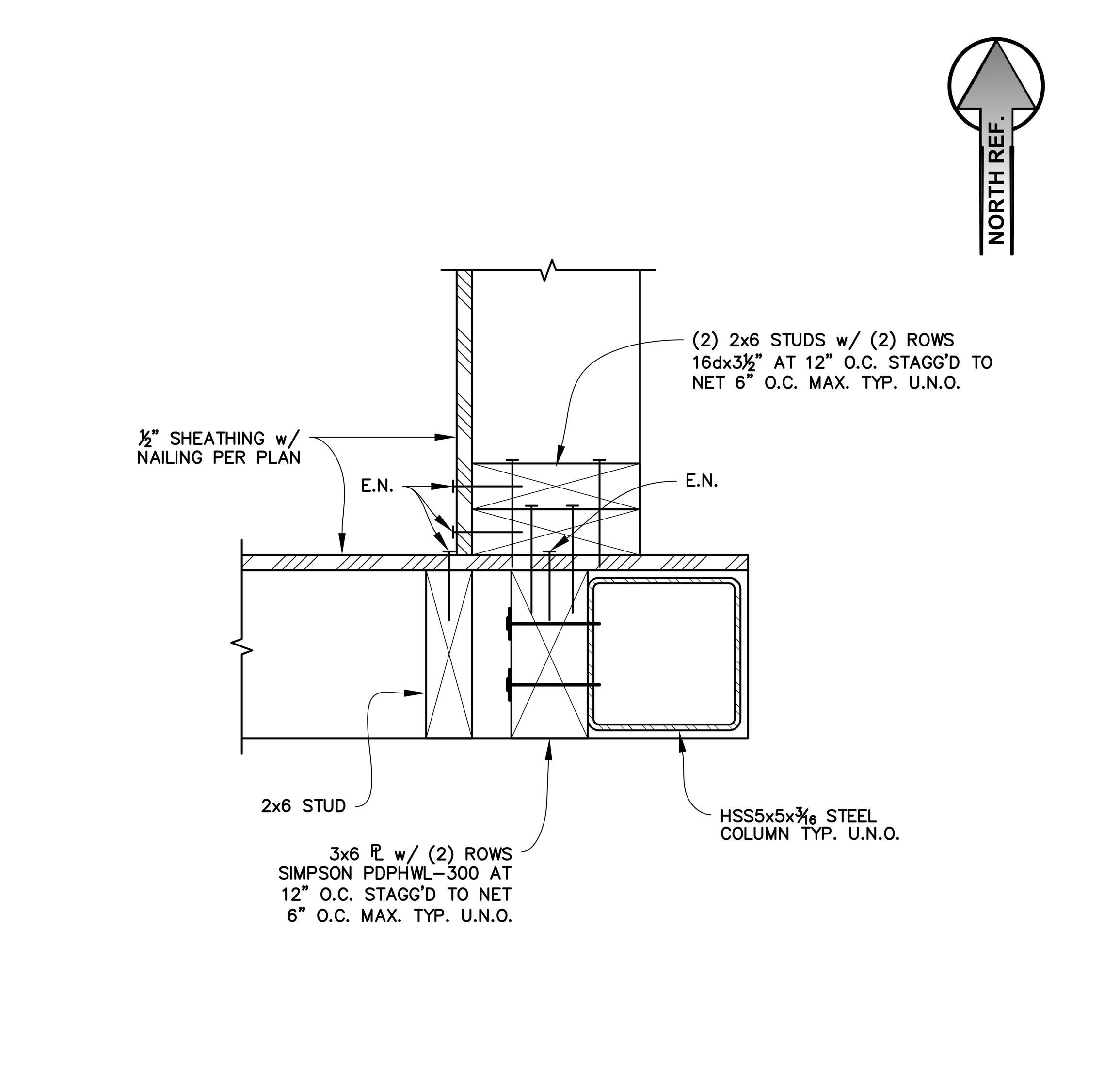
PARTIAL PLAN SECTION AT STEEL COLUMN — 7 / S4.3

SCALE: 3" = 1'-0" 1309DT49



TYP. WIDE FLANGE BEAM TO CONC. WALL CONNECTION — 11 / S4.3

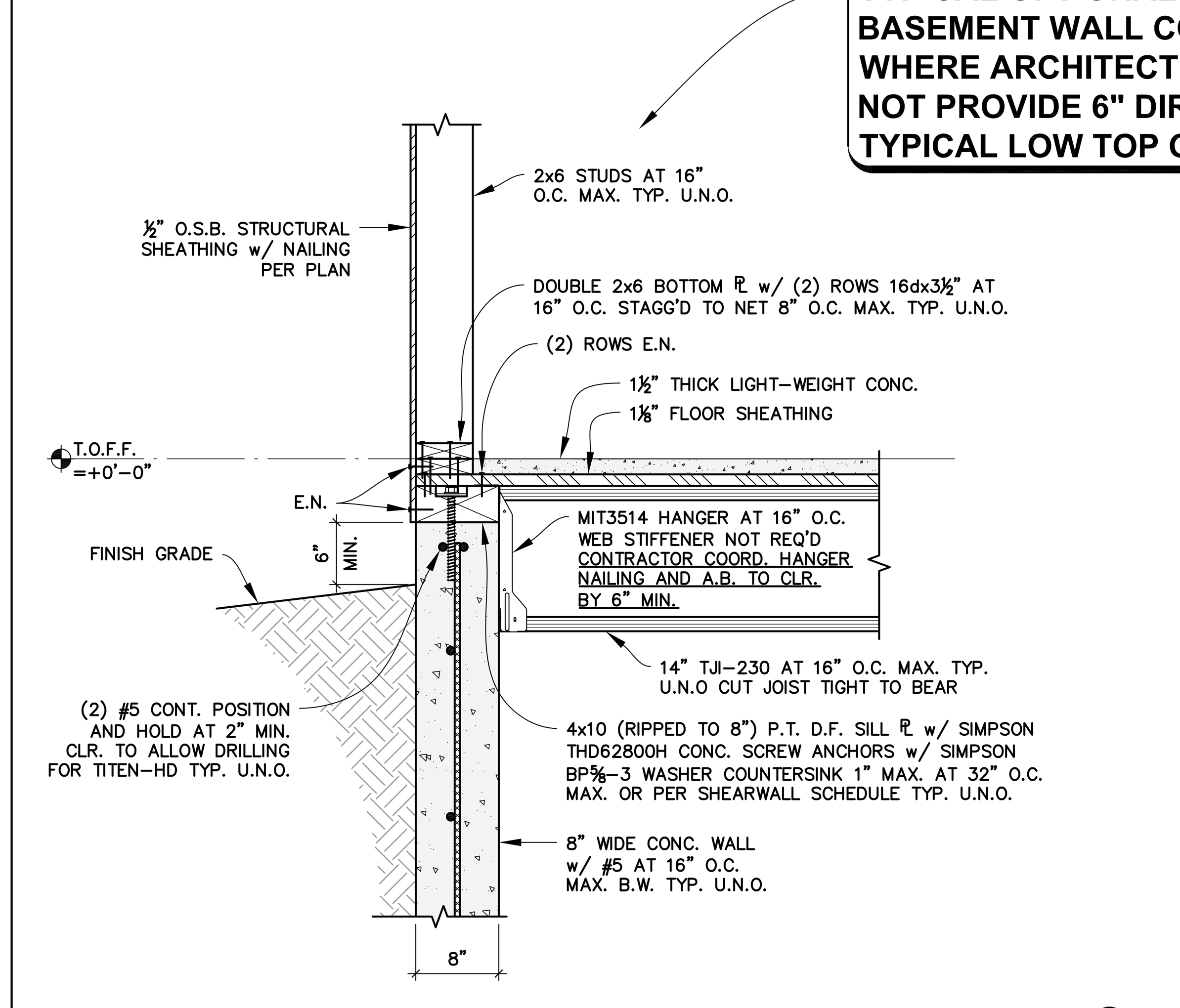
SCALE: 1½" = 1'-0" 1309DT58



PARTIAL PLAN SECTION AT STEEL COLUMN — 8 / S4.3

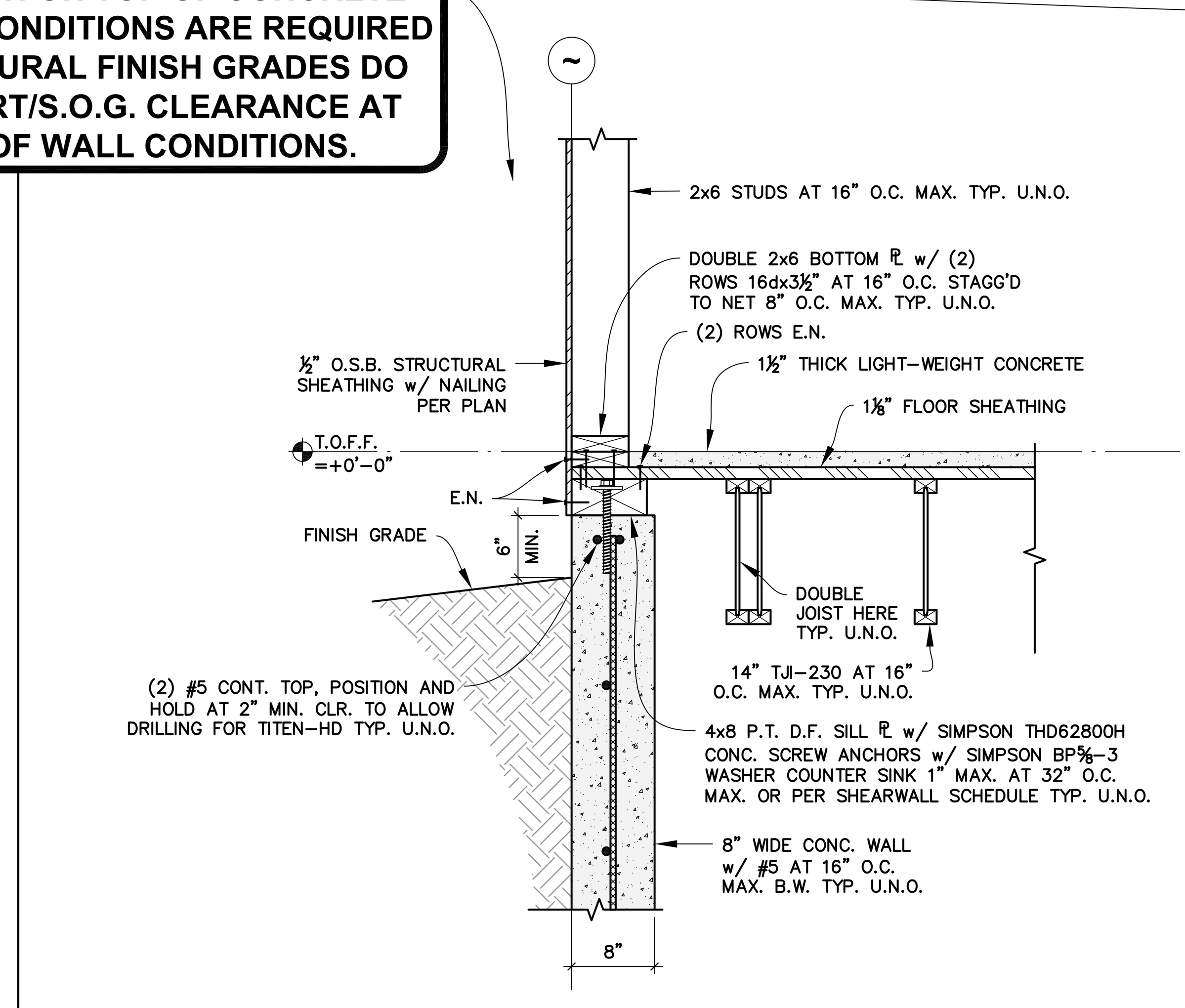
SCALE: 3" = 1'-0" 1309DT51

TYPICAL OPTIONAL HIGH TOP OF CONCRETE BASEMENT WALL CONDITIONS ARE REQUIRED WHERE ARCHITECTURAL FINISH GRADES DO NOT PROVIDE 6" DIRT/S.O.G. CLEARANCE AT TYPICAL LOW TOP OF WALL CONDITIONS.



OPTIONAL JOIST PERPENDICULAR TO CONC. WALL — 9 / S4.3

SCALE: 1" = 1'-0" 1309DT56



OPTIONAL JOIST PARALLEL TO CONC. WALL — 10 / S4.3

SCALE: 1" = 1'-0" 1309DT57

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May 27, 2009

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STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST. #425 RENO, NV 89501
PHONE: (775)322-2723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

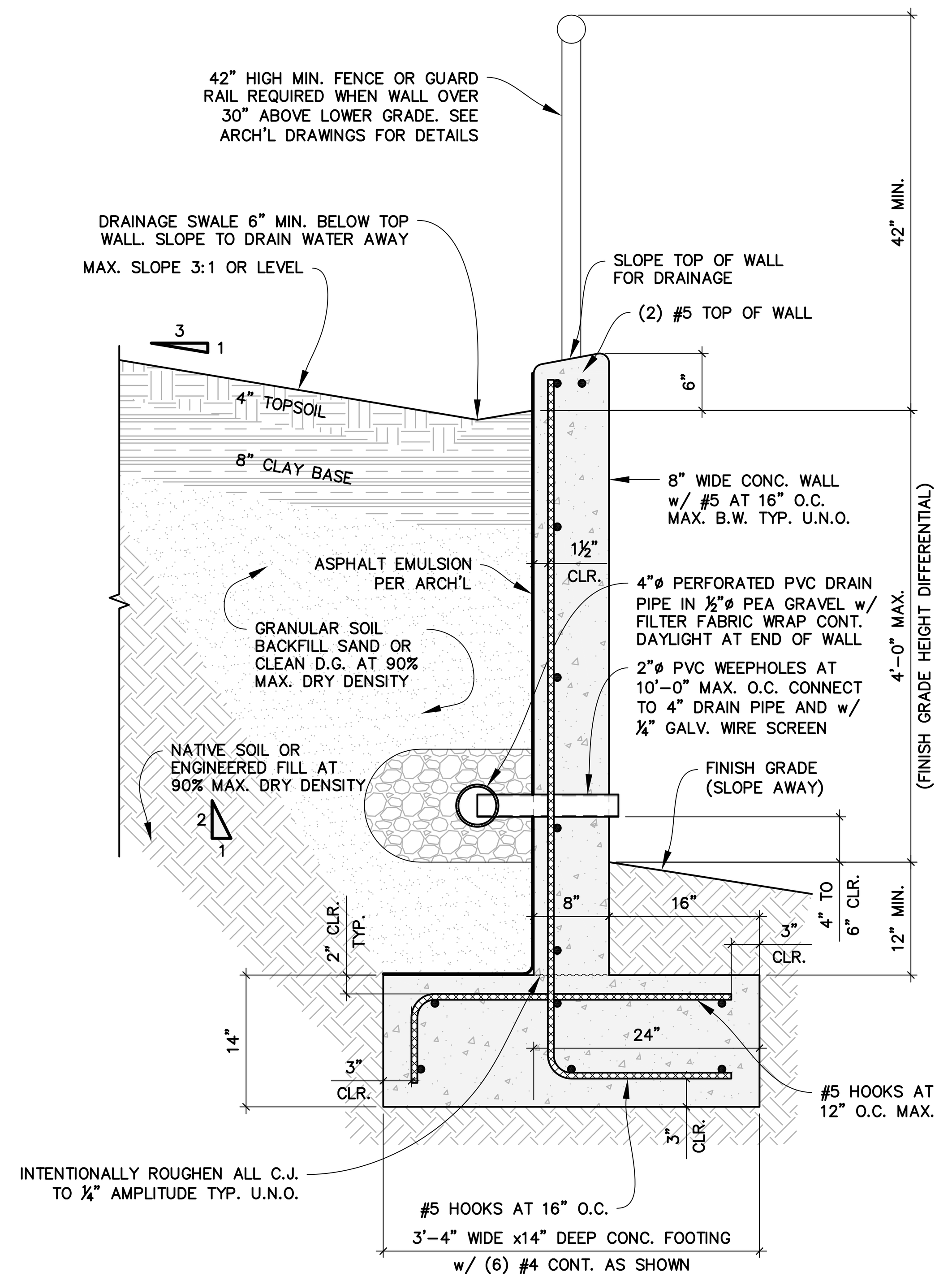
Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS: 08/31/2009 ARCH'L COORD. 09/03/2009 BUILDING DEPT. 09/10/2009 BUILDING DEPT.

DATE: 05/27/2009 SCALE: AS SHOWN DRAWN: BLW DESIGNED: TRH/KNE CHECKED: KNE JOB NO. 1309

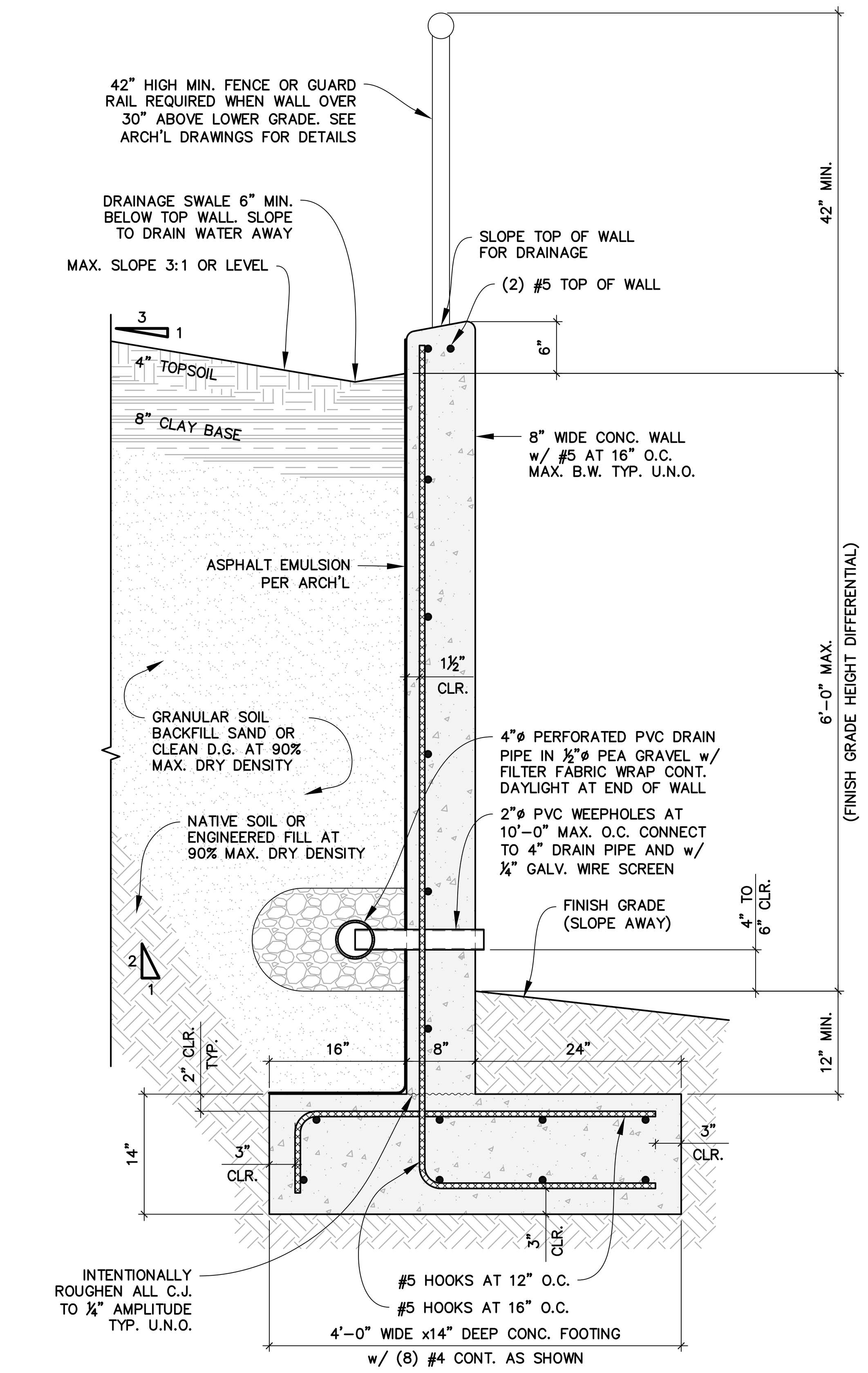
SHEET TITLE: STRUCTURAL ROOF FRAMING DETAILS

SHEET NO. S4.3 OF 15 SHEETS



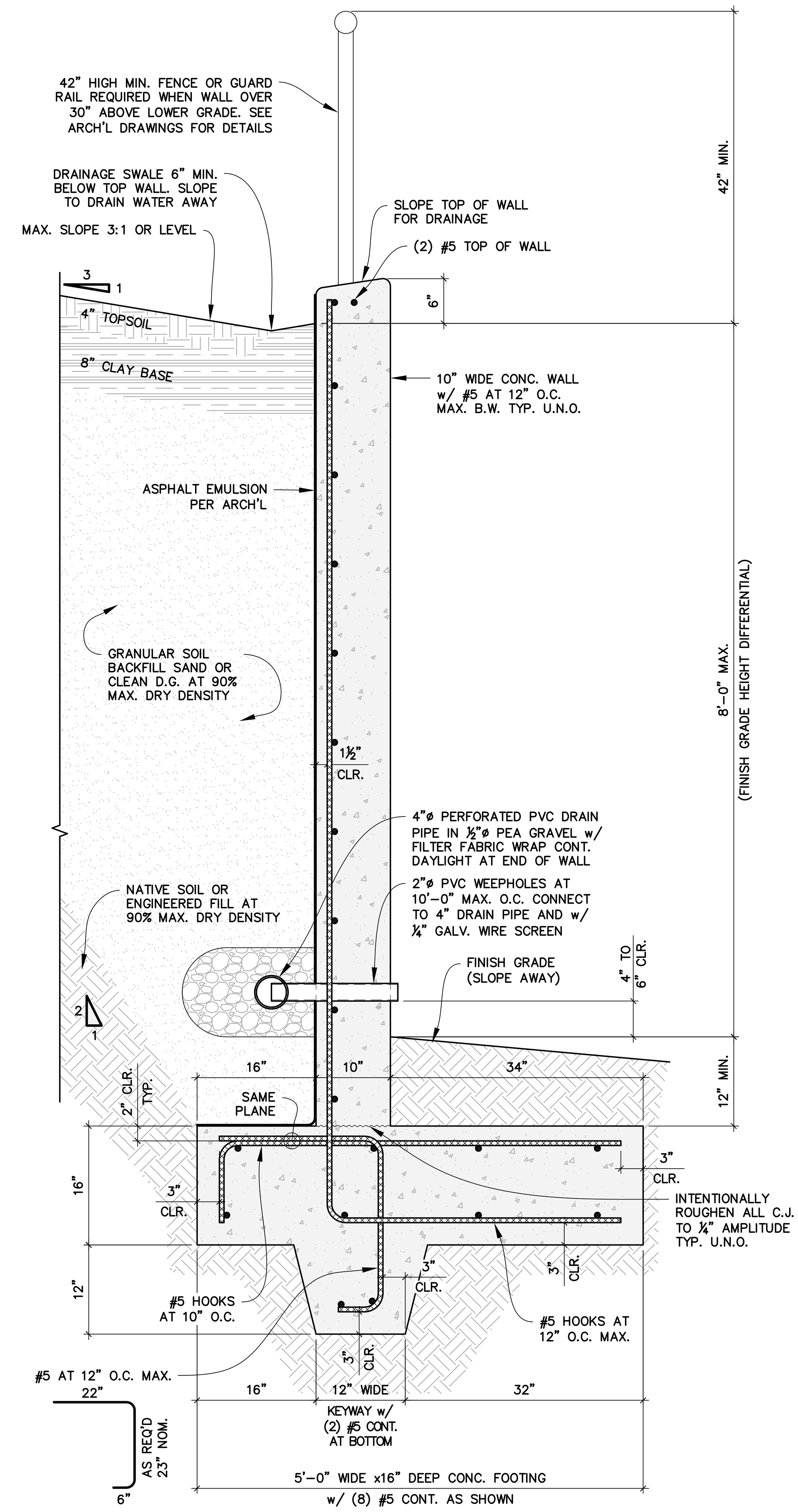
TYP. SITE CONC. RETAINING WALL (4'-0" MAX Δ HEIGHT) 1 / S4.4

SCALE: 1" = 1'-0" 1309DT55



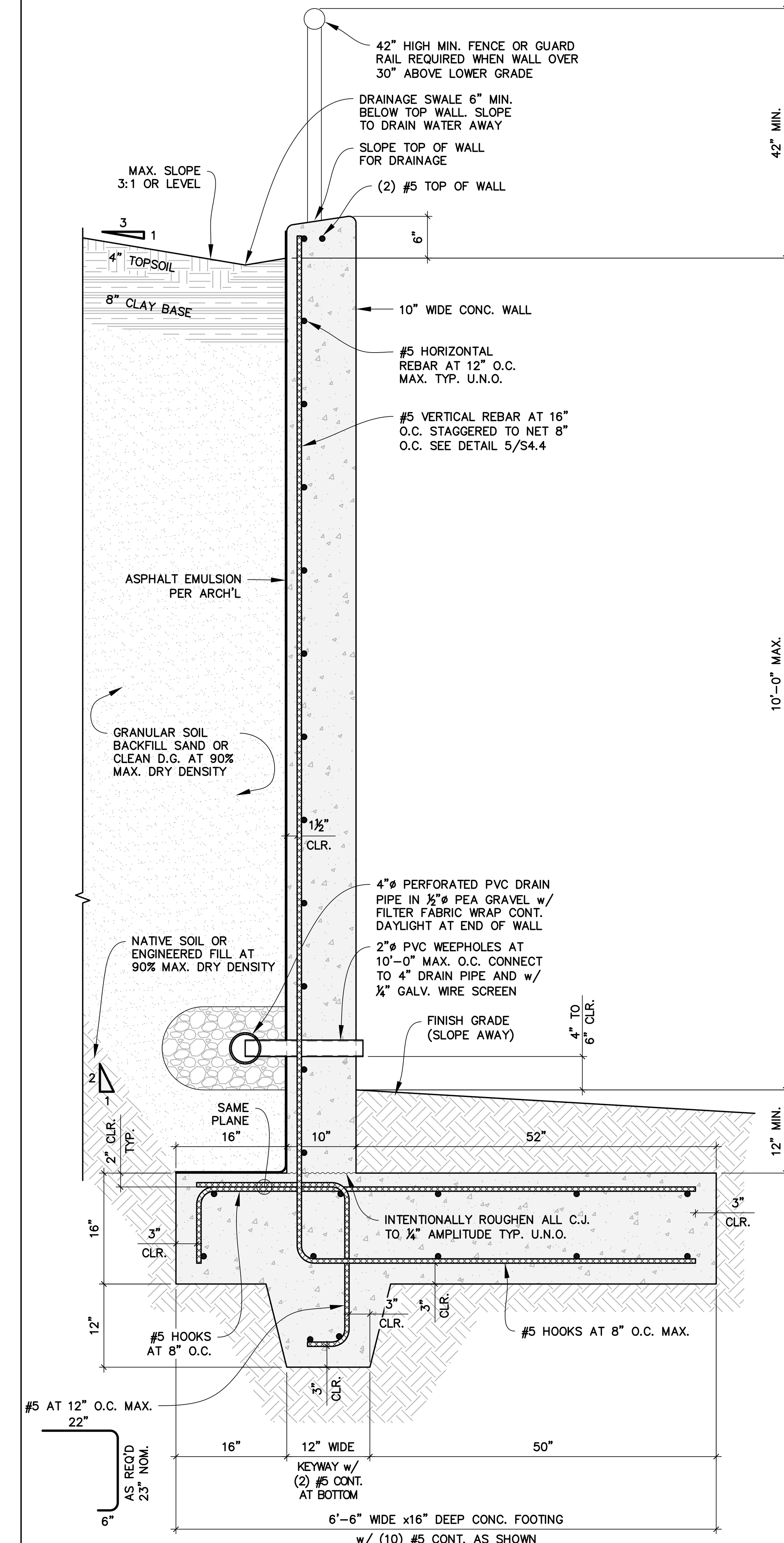
TYP. SITE CONC. RETAINING WALL (6'-0" MAX Δ HEIGHT) 2 / S4.4

SCALE: 1" = 1'-0" 1309DT55



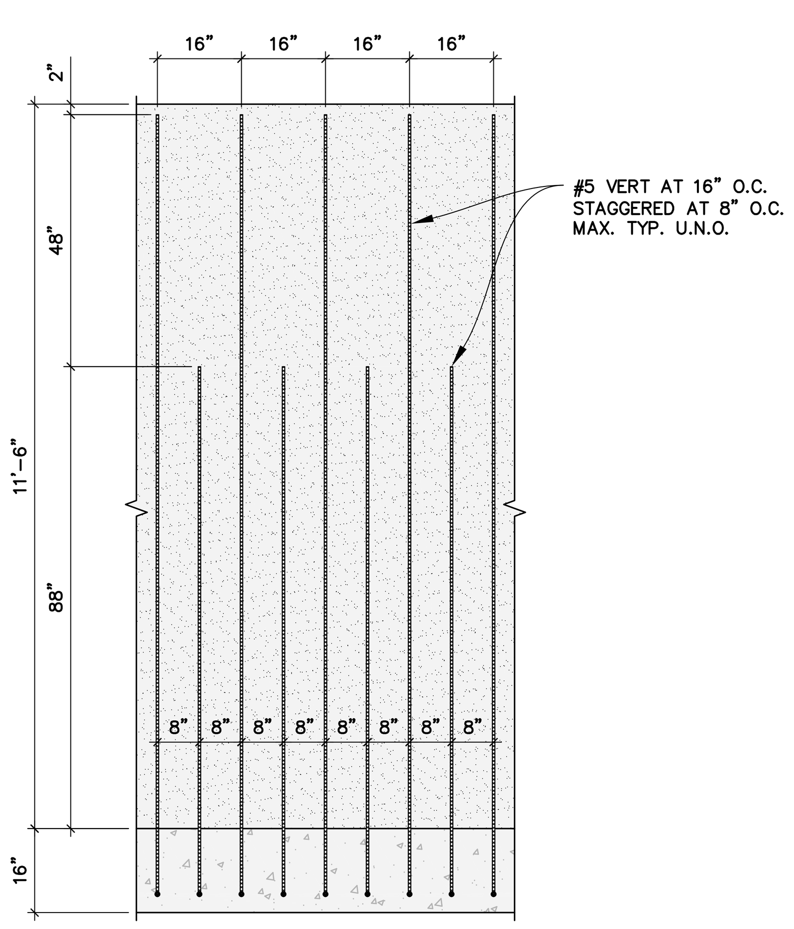
TYP. SITE CONC. RETAINING WALL (8'-0" MAX Δ HEIGHT) 3 / S4.4

SCALE: 1" = 1'-0" 1309DT55



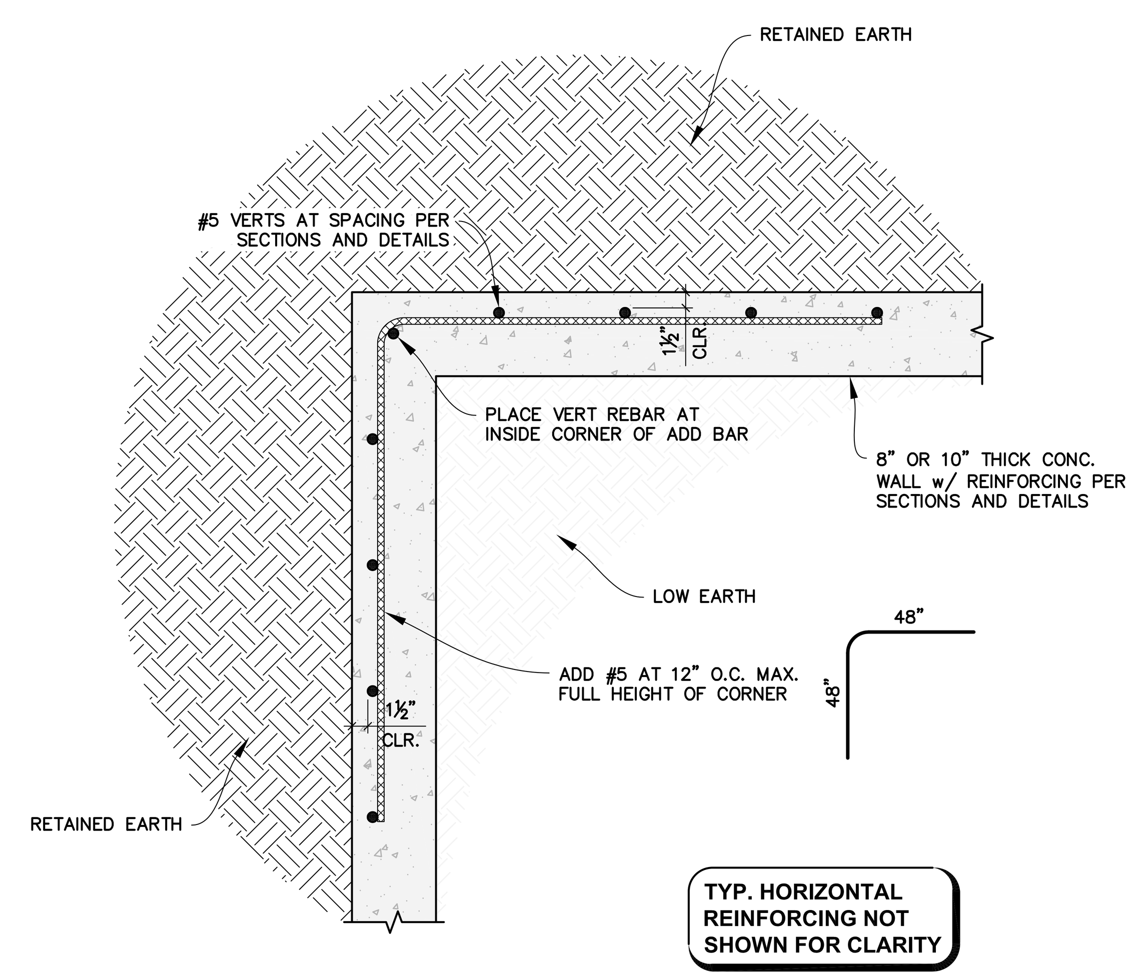
TYP. SITE CONC. RETAINING WALL (10'-0" MAX Δ HEIGHT) 4 / S4.4

SCALE: 1" = 1'-0" 1309DT55



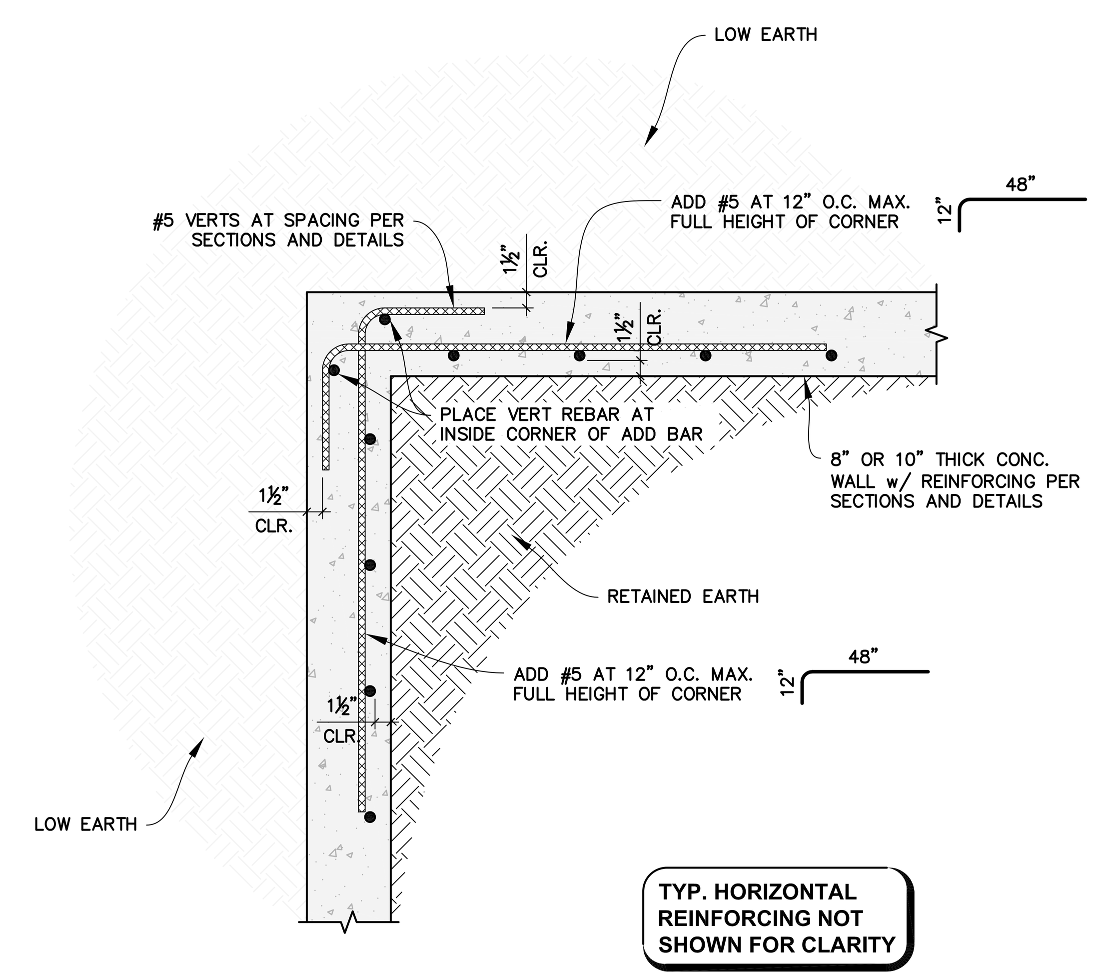
TYP. VERT. REINF. AT 10'-0" MAX. Δ HEIGHT RETAINING WALL 5 / S4.4

SCALE: ½" = 1'-0" 1309DT59



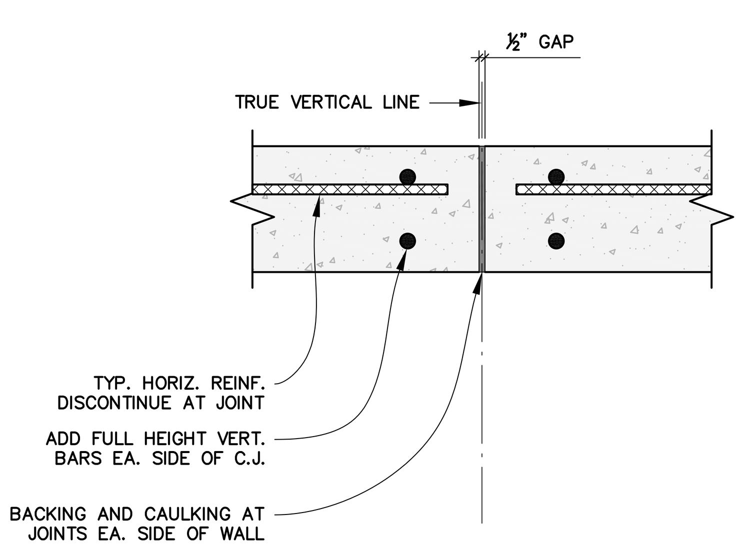
TYP. SITE RETAINING WALL INSIDE CORNER ADD BARS DETAIL 6 / S4.4

SCALE: 1" = 1'-0" 1309DT60



TYP. SITE RETAINING WALL OUTSIDE CORNER ADD BARS DETAIL 7 / S4.4

SCALE: 1" = 1'-0" 1309DT61



TYPICAL CONC. RETAINING WALL CONTROL JOINT 8 / S4.4

SCALE: 1½" = 1'-0" 1309DT62

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August 31, 2009

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KIRK N. ELLIS & ASSOCIATES
STRUCTURAL & CIVIL ENGINEERS, LTD.
245 E. LIBERTY ST. #425 RENO, NV 89501
PHONE: (775)322-7723 * FAX: (775)322-5372
E-MAIL: KNEASSOC@SBCGLOBAL.NET

PROFESSIONAL ENGINEER STATE OF NEVADA — KIRK N. ELLIS — CIVIL STRUCTURAL — No. 4398 — 08/31/2009

Steven and Jackie Vicks Residence
2445 Faretto Lane
Washoe County, NV 89511

REVISIONS:
1. 08/31/2009 ARCH'L COORD.
2. 09/01/2009 BUILDING DEPT.
3. 09/10/2009 BUILDING DEPT.

DATE	08/31/2009
SCALE	AS SHOWN
DRAWN	BLW
DESIGNED	KNE
CHECKED	KNE
KNE JOB NO.	1309

SHEET TITLE: STRUCTURAL SITE RETAINING WALL SECTIONS AND DETAILS

SHEET NO. S4.4 OF 15 SHEETS